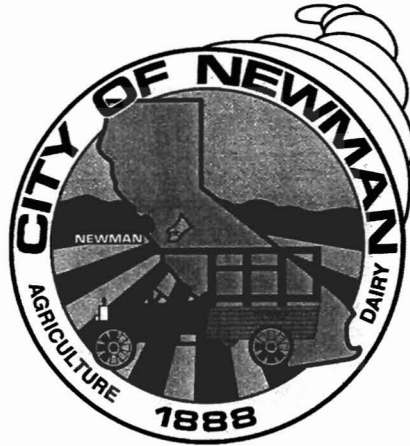




CITY OF NEWMAN

IMPROVEMENT STANDARDS

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IMPROVEMENT STANDARDS SPECIFICATIONS

TABLE OF CONTENTS

SECTION 1 - GENERAL

1.1	IMPROVEMENT STANDARDS	1-1
1.2	DEFINITIONS	1-1
1.3	PLANS AND SPECIFICATIONS	1-3
1.4	SUPPLEMENTAL INFORMATION	1-7
1.5	DEPARTMENTAL REVIEW	1-8
1.6	IMPROVEMENT AGREEMENT	1-8
1.7	SHOP DRAWINGS AND SUBMITTALS	1-8
1.8	CONSTRUCTION REVIEW	1-9
1.9	RECORD DRAWINGS	1-11
1.10	EQUIPMENT OPERATION AND MAINTENANCE SUBMITTALS	1-11

SECTION 2 – TRAFFIC CONTROL

2.1	GENERAL	2-1
2.2	TRAFFIC CONTROL	2-1
2.3	SIGNS	2-1
2.4	BARRICADES	2-2
2.5	FLASHER SUPPORTS	2-2
2.6	DELINEATORS	2-2
2.7	FLAGGER	2-2
2.8	WARNING LIGHTS	2-3
2.9	STREET CLOSURE	2-3

SECTION 3 – STREETS

3.1	GENERAL	3-1
3.2	GEOMETRIC DESIGN	3-1
3.3	STRUCTURAL DESIGN	3-2
3.4	CLEARING	3-3
3.5	EARTHWORK	3-3
3.6	SUBGRADE	3-3
3.7	AGGREGATE BASE	3-4
3.8	ASPHALT CONCRETE	3-4
3.9	ASPHALT PAINT BINDER	3-4
3.10	HEADER BOARDS	3-4

3.11	CONCRETE	3-5
3.12	CURBS, GUTTERS AND SIDEWALKS	3-5
3.13	DRIVEWAY APPROACHES-GENERAL.....	3-6
3.14	RESIDENTIAL DRIVEWAYS.....	3-7
3.15	COMMERCIAL DRIVEWAYS.....	3-7
3.16	ALLEYS	3-7
3.17	VALLEY GUTTERS	3-7
3.18	CURB RAMPS.....	3-7
3.19	RAISING UTILITY BOXES	3-8
3.20	TESTING	3-8
3.21	ACCESS CONTROL WALLS	3-8
3.22	PAVING ADJACENT TO EXISTING STREETS	3-8

SECTION 4 - LIGHTING

4.1	GENERAL	4-1
4.2	REGULATIONS AND CODES	4-1
4.3	EXCAVATION AND BACKFILL	4-1
4.4	CONDUIT	4-1
4.5	PULL BOXES... ..	4-1
4.6	FOUNDATIONS	4-2
4.7	ANCHOR BOLTS	4-2
4.8	CONDUCTORS	4-2
4.9	WIRING	4-2
4.10	POLES	4-2
4.11	ARMS	4-3
4.12	ELECTROLIERS	4-3
4.13	PHOTOELECTRIC CELLS	4-3
4.14	TESTING	4-3
4.15	RECORD DRAWINGS	4-3

SECTION 5 - WATER

5.1	GENERAL	5-1
5.2	DESIGN SUBMITTALS	5-1
5.3	DESIGN FLOW	5-1
5.4	PIPE DESIGN	5-2
5.5	PIPE	5-4
5.6	FITTINGS	5-4
5.7	VALVES AND VALVE BOXES	5-5
5.8	SERVICES	5-5
5.9	SAMPLING STATIONS	5-6
5.10	TRENCH EXCAVATION	5-6
5.11	WATER IN TRENCH	5-7

5.12	LAYING AND JOINTING OF PIPE	5-7
5.13	HANDLING OF PIPE ACCESSORIES	5-7
5.14	SERVICE INSTALLATION	5-7
5.15	FIRE HYDRANTS	5-8
5.16	CONNECTIONS WITH EXISTING WATER LINES	5-8
5.17	INSPECTION	5-8
5.18	BEDDING AND BACKFILL OF TRENCHES	5-10
5.19	RESTORING SURFACE	5-10

SECTION 6 - SEWER

6.1	GENERAL	6-1
6.2	DESIGN SUBMITTALS	6-1
6.3	DESIGN	6-1
6.4	PIPE FOR SEWER MAINS	6-4
6.5	SERVICES	6-4
6.6	MANHOLES	6-5
6.7	LIFT STATIONS	6-5
6.8	CONTROLS	6-6
6.9	DISCHARGE PIPING	6-7
6.10	FORCE MAINS	6-7
6.11	TRENCH EXCAVATION	6-8
6.12	WATER IN TRENCH	6-9
6.13	LAYING AND JOINTING OF PIPE	6-9
6.14	LATERAL CONNECTIONS	6-9
6.15	BACKFILL	6-10
6.16	CLEANING AND FLUSHING	6-10
6.17	INSPECTION	6-11
6.18	CLOSED CIRCUIT TV INSPECTION	6-11
6.19	GREASE INTERCEPTORS	6-12

SECTION 7 – STORM DRAINAGE

7.1	GENERAL	7-1
7.2	SUBMITTALS	7-1
7.3	FLOW RATES	7-1
7.4	PIPE DESIGN	7-2
7.5	DRAIN INLETS	7-4
7.6	MANHOLES	7-4
7.7	TRENCH EXCAVATION	7-4
7.8	WATER IN TRENCH	7-5
7.9	LAYING AND JOINTING OF PIPE	7-5
7.10	BACKFILL	7-6
7.11	CLOSED CIRCUIT TV INSPECTION	7-6

7.12	DETENTION BASINS	7-7
7.13	PUMP STATION	7-8
7.14	TRASHRACKS	7-9
7.15	ACCESS	7-10

SECTION 8 – MISCELLANEOUS

8.1 PARKING

8.1.1	GENERAL	8-1
8.1.2	PARKING DESIGN GUIDE	8-1
8.1.3	HANDICAP PARKING REQUIREMENTS	8-2
8.1.4	PAVEMENT	8-3
8.1.5	STORM DRAINAGE	8-3

8.2 CONCRETE WASHOUT

8.2.1	GENERAL	8-5
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8.3 MONUMENTS

8.3.1	GENERAL	8-6
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SECTION 1

GENERAL

1.1 IMPROVEMENT STANDARDS

These Improvement Standards have been prepared by the City of Newman to aid all persons engaged in the design and construction of public works facilities within the city limits of Newman. (Portions of these standards also apply to private work within the city limits which affect public health and safety.)

The data contained herein is not intended for use as Contract Documents for contracts between the City and a Contractor, or for contracts between a subdivider or private person and a Contractor. Rather, separate Contract Documents must be prepared for each project, with each such document containing a Special Provisions section applicable to that particular project.

Design Engineers and Contractors working in the City of Newman should be familiar with these Improvement Standards. All Plans and Specifications for construction of improvements to be accepted by the City shall be prepared in accordance with these standards. Improvements shall be constructed in accordance with these standards.

These Improvement Standards do not cover all the work which may require acceptance by the City. Work which is not covered by these standards shall be designed in accordance with generally accepted engineering principles. It is recommended that the criteria for work which is to be accepted by the City and which is not covered by these Improvement Standards be reviewed with the City Engineer prior to the actual design to establish design criteria. (Design criteria will be based on current codes and regulations applicable to the work and the accepted principles of engineering. The City Engineer may require the Design Engineer to submit engineering calculations for the work.)

Nothing in these Improvement Standards is intended to reduce or modify applicable Federal, State or local laws, nor to create a standard to be applied retroactively to existing improvements except where specifically noted.

All work performed within State Highway or railroad rights-of-way shall be in accordance with the governing agency standards.

1.2 DEFINITIONS

When used for the construction of any improvements within the City of Newman, the appropriate definitions and terms listed in Section 1 of the State Standards shall apply, with the following modifications:

CITY. The City of Newman or any persons to whom the power of the City has been delegated.

CITY ENGINEER. The City Engineer of the City of Newman acting either directly or through properly authorized agents, such agents acting within the scope of the particular duties delegated to them.

CITY MANAGER. The City Manager for the City of Newman.

CONTRACTOR. An individual, firm, corporation, partnership or association duly licensed by the State of California who does work for City acceptance.

DESIGN ENGINEER. The registered engineer licensed by the State of California responsible for preparation of plans and specifications for work to be accepted by the City.

DIRECTOR. The Director of Public Works for the City of Newman.

ENGINEER. The registered engineer licensed by the State of California in responsible charge of the project. (Normally this is the Design Engineer.)

ENGINEER'S ESTIMATE. The list of estimated quantities of work and the estimated cost to perform the work.

IMPROVEMENT PLANS. Plans of proposed improvements prepared by the Design Engineer, after they have been approved by the City Engineer.

IMPROVEMENT STANDARDS. The Improvement Standards of the City of Newman which include design standards, construction standards and specifications and standard details.

PRODUCT DATA. All illustrations, standard schematics, performance charts, instructions, brochures, diagrams and other information furnished by the CONTRACTOR to illustrate a material, product or system for some portion of the work.

SHOP DRAWINGS. All drawings, diagrams, schedules and other data which are specifically prepared for the work by the Contractor, a subcontractor, manufacturer, supplier or distributor, which illustrates how specific portions of the work shall be fabricated or installed.

SPECIFICATIONS. Directions, provisions and requirements prepared by the Design Engineer pertaining to performance of the work.

STATE STANDARDS. Current edition of the State of California Standard Plans and Specifications as issued by the Department of Transportation (where "State" is referred to, substitute with "City of Newman").

1.3 PLANS AND SPECIFICATIONS

All Improvement Plans, Specifications and Special Provisions shall comply with the requirements of the approved or conditionally approved development plan and/or tentative map and these Improvement Standards.

Improvement Plans submitted for preliminary review to the City or City Engineer shall be stamped, need not be signed, but should be complete. The following details and supplemental information shall be shown on plans submitted:

1. GENERAL REQUIREMENTS

- a. Improvement Plans shall show all existing activities and all improvements to be constructed.
- b. Size: The size of the Improvement Plan sheets shall be 24" x 36" or 22" x 34".
- c. Scale: The scales selected shall be sufficient to clearly show all required details when reproduced on Blueline. Preferred vertical scales are 1" = 4' in reasonably flat areas.
- d. Title Block: Each sheet within the set shall have a title block showing the project's name, sheet title, date of drawing and revisions, scale of drawings, sheet number, and the Design Engineer's name, registration number, expiration date of registration, and signature.
- e. Vertical Control: All elevations indicated in the Improvement Plans shall be based on City datum. Benchmark location, number, and elevation shall be shown on the drawings.
- f. Orientation and Stationing: Insofar as practical, the plans shall be arranged so that the North shall be at the top or right edge of the sheet. The stationing on plan and profile sheets shall read from left to right or from bottom to top.

2. TITLE SHEET

- a. Title sheets shall be prepared for Improvement Plans exceeding two sheets per set, and shall show the following:

- The entire project, drawn at a suitable engineering scale, including existing and proposed street names and lot numbers. Surrounding lot lines shall be shown within a minimum distance of 50 feet of the project.
- Vicinity map and north arrow.
- Index of sheets.
- Legend of symbols.
- Location, description and elevation of the reference City Benchmark as well as any temporary benchmark used for the project.
- Name, address, telephone number and designated agent of any agency whose facilities will be installed, utilized, interfered with, or crossed as part of the improvements, as well as a signature block for their approval. Where construction requires encroachment permits by other agencies, copies of signed encroachment permits together with evidence of any required insurance shall be submitted.
- Name, address and telephone number of the developer or his authorized representative.
- Signature block for approval by the City Engineer as follows:

APPROVED BY THE CITY ENGINEER, CITY OF
NEWMAN REVIEWED FOR CONFORMANCE WITH
CITY OF NEWMAN REQUIREMENTS ONLY. SINCE
PLANS WERE PREPARED BY OTHERS, NO
RESPONSIBILITY FOR THE ADEQUACY OF THE
DESIGN IS EXPRESSED.

BY _____ DATE _____

b. The following notes shall be placed on the title sheet:

- This set of Improvement Plans is valid for construction purposes only after being signed by the City Engineer and upon issuance of permits.

- All Contractors and Subcontractors involved in the construction of this project shall attend a pre-construction conference arranged by the Developer at the Department of Public Works for construction and inspection coordination prior to commencement of any on-site construction activities.
- The current Newman Department of Public Works Improvement Standards that have been referenced in these plans shall be considered as part of these plans.

c. The following shall not be included in the title sheet:

- Quantities List.

3. TOPOGRAPHY SHEET

A topographical survey sheet shall be included in the improvement plan set and shall show spot elevations at an appropriate interval, fences, structures, pipelines, ditches, utility poles, trees, driveways, roads, pavement, wells, rights-of-way, easements, etc., and their disposition. Disposition of existing facilities may be indicated on the topographical survey sheet by reference to appropriate sheets within the Improvement Plans.

4. GRADING AND DRAINAGE SHEET

A grading and drainage sheet shall be included in the Improvement Plans set and shall show the following:

- a. A typical lot grading detail. (Specific lots may be required to provide an independent grading plan.)
- b. Proposed lot corner elevations as well as any elevation differential between the project boundaries and the adjoining properties.
- c. Gutter or ditch flow arrows, slopes, and grade break elevations.
- d. Storm drainage pipes, manholes, valley gutters, and catch basins.
- e. Detention basin location and details.

- f. Location and height of any retaining walls or retaining fences. Concrete or masonry retaining walls shall be provided where the difference in grade at property line exceeds 12 inches. For grade differentials of 12 inches or less, a grading strip with a 6 horizontal:1 vertical slope may be utilized.

5. UTILITIES SHEET

A utilities sheet shall be included in the set of Improvement Plans and shall show street lights, conduits, pullboxes and connection points to the serving companies utilities, fire hydrants, water lines, valves, blowoffs, sanitary sewer lines, manholes and clean outs, sewer and water service locations, water wells, PUE's, driveways, centerline monuments, street signs, etc. If available prior to approval of improvement plans, power lines, gas lines, TV cable lines, telephone lines and mail box locations shall be provided. If not, these utilities shall be shown on the required Record Drawings. Utility company Record Drawings may be accepted in lieu of required Record Drawings provided sufficient detail is provided to coordinate the electrical, gas, telephone and TV cable line locations with other utilities as determined by the Director of Public Works.

6. PLAN AND PROFILE SHEET

A plan and profile sheet shall be included in the set of Improvement Plans showing the existing and proposed profiles of all roadways and other improvements in public rights-of-way. This sheet shall show elevations, grade breaks, vertical curves, slope, road stationing, storm drainage lines, water lines, sewer lines, irrigation lines and any areas of possible conflict between underground utilities. Indicate length and type of all sewer and storm drain pipes and catch basins. Show elevations of pipe inverts in manholes and catch basins. Indicate length and type of all water system pipes and locations of valve junctions.

7. DETAIL SHEET

A detail sheet shall be included in the set of Improvement Plans showing typical construction details. Construction details included in the Improvement Standards, that are applicable to the work, shall be reproduced and included on the detail sheet(s).

8. COMBINING REQUIRED SHEETS

Sheets may be combined or omitted depending on the complexity of the project.

1.4 SUPPLEMENTAL INFORMATION

The following supplemental information shall be submitted with the initial sets of Improvement Plans unless otherwise indicated in these Standards.

1. GEOTECHNICAL INVESTIGATION REPORT

The required geotechnical report shall be prepared by a Civil or Geotechnical Engineer legally authorized to practice in the State of California. Three (3) copies shall be submitted. The geotechnical report shall include:

- a. The results of "R" value tests taken in the project site and a recommendation by the Civil or Geotechnical Engineer for design "R" values for road pavement sections.
- b. Depths to groundwater measurement or other records of depth to groundwater. Civil or Geotechnical Engineer's recommendations for elevations of roads above groundwater.
- c. Trench backfill recommendations.
- d. Recommendations for subgrade conditioning underneath concrete curbs, gutters and sidewalks to minimize post-construction movement of these improvements due to movement of underlying soils.

2. MISCELLANEOUS CALCULATIONS

Submit calculations for pavement structural section determination plus any calculations used in the design of any retaining walls or other miscellaneous items not covered in these standards, such as domestic water flow, fire flow, sewage flow, and storm drainage calculations.

3. QUANTITIES LIST AND ENGINEER'S ESTIMATE

Quantities list and engineer's estimate shall show estimated costs, descriptions and total costs of each item of work. Engineer's

estimate shall be separated into items that deal with storm drainage, domestic water systems, sanitary sewers, etc., and shall include a separate 10% contingency for the total value of work to be performed.

4. PRODUCT SPECIFICATIONS

When a product is mentioned in the Improvement Plans such as pumps, motors, street lights, etc., the Design Engineer shall submit the manufacturer's specifications upon request.

1.5 DEPARTMENTAL REVIEW

The Design Engineer shall submit for review to the Planning Department the number of sets of Improvement Plans requested, together with the initial plan check fee. When corrections are required, one set will be returned to the Design Engineer showing required changes. The Design Engineer shall then resubmit the corrected Improvement Plans for review. The number of sets resubmitted will be determined by the City Engineer. In order to reduce the man-hours required in checking resubmittals of Improvement Plans, the Design Engineer shall highlight, in yellow, all changes that have been made on one of the required Blueline sets resubmitted. The City will make every reasonable effort to provide all pertinent comments and identify all necessary corrections during the first improvement plan review. However, the Design Engineer's changes in response to the comments may themselves require further comments and corrections, and may necessitate changes in other areas of the design. Thus, the Design Engineer is cautioned not to assume all the changes have been identified during the first submittal review cycle.

After all corrections have been made to the satisfaction of the City Engineer and other departments, the entire set of originals shall be signed by the Design Engineer and a number of plan sets as determined by the City Engineer submitted to the City for approval.

1.6 IMPROVEMENT AGREEMENT

The Developer shall enter into an Improvement Agreement with the City for all of the improvements shown on the approved Improvement Plans. The Improvement Agreement shall be fully executed and required bonds or securities submitted prior to any construction being allowed to begin.

1.7 SHOP DRAWINGS AND SUBMITTALS

The Contractor shall provide the Design Engineer with Shop Drawings and Product Data required by the Specifications or Improvement Plans or as otherwise submitted for custom design work including but not limited to pumping stations,

bridges and other structures. The Design Engineer shall promptly review all Shop Drawings and Product Data submitted and return reviewed Shop Drawings and Product Data directly to the Contractor; two copies of favorably reviewed Shop Drawings and Product Data shall be sent directly to the City Engineer for construction monitoring.

The Contractor shall also submit to the Design Engineer for review engineering calculations as required by the Specifications or Improvement Plans or to support the Shop Drawing and Product Data submittals. The Design Engineer shall promptly review these items and return them directly to the Contractor; two copies of favorably reviewed calculations shall be sent directly to the City Engineer.

The City Engineer may request to review Shop Drawings or Product Data for City acceptability of major components and assemblies. If such a request is made, the Design Engineer shall submit the requested Shop Drawings or Product Data to the City Engineer following favorable review by the Design Engineer. The City Engineer shall promptly review the requested submittals, make a determination on acceptability and return the submittals to the Design Engineer.

The Contractor shall submit to the Engineer all proposed substitutions of items in these Improvement Standards where an "or equal" option is not explicitly permitted. The City Engineer shall promptly review these proposed substitution submittals, make a determination on whether or not the proposed substitutions are acceptable, and return the reviewed submittals directly to the Contractor.

1.8 CONSTRUCTION REVIEW

All work accomplished and all materials furnished under these Improvement Standards shall be subject to the inspection and approval of the Director. Such inspection and approval of work and materials shall not relieve the Contractor of any of his obligations to complete the work specified. Work and materials not meeting these requirements shall be corrected.

The Director shall have access to the work at all times and shall be furnished every reasonable opportunity for ascertaining that the methods, materials and workmanship are in accordance with the requirements and intent of these Improvement Standards. The Contractor or his authorized agent shall be in charge of and responsible for all phases of work while it is in progress.

The Director shall be notified in writing by the Contractor at least two working days prior to beginning any of the stages of work listed below and shall be notified when each of the stages has been completed. Subsequent stages shall not begin until the Director has approved the previous stage.

1. Placement of culvert pipes, storm drains, sanitary sewer, waterlines and other utilities. Upon completion of the backfill of all trenches in the public rights-of-way, the Geotechnical firm providing compaction testing shall furnish the City a letter, that all trench backfill was accomplished per the recommendations of the Geotechnical Report and met the minimum compaction requirements per these Improvement Standards.
2. Placement of any layer of subbase, base or surfacing material, including the preparation of the subgrade for streets and roads, curb, gutters and sidewalk.
3. Installation of reinforcing steel and preparation of structural subgrade.
4. Placement of concrete
5. Placement of structure backfill material.
6. Testing and start-up.

In addition to the above, the Contractor shall notify the Director in writing whenever improvement work is to be performed on Saturdays, Sundays or holidays or during hours of the day when such work is normally not performed so that inspection may be provided.

The Contractor shall give the Director sufficient notice regarding proposed sources of materials to be used in the work so that such tests and inspections as the Director deems necessary can be performed to determine that the materials comply with these standards.

All tests of materials and work to determine compliance with these standards shall be in accordance with City approved methods and procedures. If required, the Contractor shall furnish to the Director, without charge, samples of all materials to be used in the work. Samples of material from which tests are to be made shall be taken under the supervision of the Director by a recognized laboratory.

The Contractor shall be responsible for coordinating all required testing, shall notify the Director when testing is to be performed and shall be responsible for the testing laboratory submitting results in a timely manner. Subsequent work shall not be started until all tests pass and written reports are filed with the Director.

The Developer, Contractor or Utility shall inspect and repair all defective work done in the public rights-of-way for a period of one year from the date the work is

accepted by the City, or if subsequent repairs are required, one year from the date the repairs are complete and acceptable.

1.9 RECORD DRAWINGS

Prior to the City's acceptance of the improvements, the Design Engineer shall compile and submit a set of Record Drawings showing final improvement details, corrected improvement elevations and locations, as well as any changes that occurred during construction. Record Drawings shall consist of a bond copy, an electronic (PDF) copy of the record drawings along with an electronic (AutoCAD) file for all utilities (water, sewer and storm drain lines with sizes, manholes, valves, catch basins, etc.) street rights-of-way, street names and lot lines. (The AutoCAD file will be utilized for updating utility base maps.).

Original data that has been superseded shall be crossed out, but not eradicated. All utilities that could not be shown on the construction plans shall be drawn on the Record Drawings. The Design Engineer shall provide final elevations of all catch basins, storm drainage pipe inverts, sewer flowline elevations at manholes and curb and gutter flowline. All lettering must be clear and legible. Extensive changes which cannot be shown clearly on an original sheet shall be drawn on a supplemental sheet. All supplemental sheets shall be signed by the Design Engineer and included as part of the Record Drawings. The Design Engineer shall sign the bond copies of the Record Drawings. The Record Drawings will be retained by the City.

1.10 EQUIPMENT OPERATION AND MAINTENANCE SUBMITTALS

Prior to the acceptance of the improvements by the City, the Developer shall compile and submit information and materials related to each maintainable piece of equipment, equipment assembly, or sub-assembly provided and dedicated to the City in conjunction with his work. This requirement generally applies to such items as pumps, lift stations, irrigation systems, and similar equipment and systems. It also includes valves, fire hydrants, and similar items not specifically listed in the Improvement Standards that have been provided on an "or equal" basis, where operation and maintenance of such items differs from listed items and replacement parts and special tools are not directly interchangeable with those for listed items.

1. OPERATION AND MAINTENANCE MANUALS

Three sets of operation and maintenance manuals shall be provided for each maintainable piece of equipment, equipment assembly or sub-assembly which covers the following subjects in detail:

General

- Names, addresses, and telephone numbers of the manufacturer, the nearest representative of the manufacturer, and the nearest supplier of the manufacturer's equipment and parts.

In addition, one or more of the following items of information shall be provided as applicable, to the satisfaction of the Director of Public Works:

Operating Instructions

- Safety precautions
- Operator prestart
- Startup, shutdown, and post shutdown procedures
- Normal operations
- Emergency operations
- Operator service requirements
- Environmental conditions

Preventive Maintenance

- Lubrication data
- Preventive maintenance plan and schedule

Corrective Maintenance

- Troubleshooting guides and diagnostic techniques
- Wiring diagrams and control diagrams
- Maintenance and repair procedures
- Removal and replacement instructions
- Spare parts and supply list
- Corrective maintenance man-hours

Appendices

- Parts identification
- Warranty information
- Personnel training requirements
- Testing equipment and special tool information

2. ROUTINE MAINTENANCE ITEMS AND SUPPLIES

Consumables, lubricants, gaskets, fuses, and similar routine maintenance items and supplies (not including fuel) sufficient for one (1) year operation.

SECTION 2

TRAFFIC CONTROL

2.1 GENERAL

This Section is intended to establish general principles of traffic control, worker protection and public safety measures to be taken in the performance of all work covered by these Improvement Standards.

No specification contained herein shall be deemed to create a legal standard of conduct or duty toward the public nor shall it limit the City in the exercise of powers conferred by law in modifying these Improvement Standards.

The requirements of the State of California, Department of Transportation, "California Manual on Uniform Traffic Control Devices", herein referred to as the California MUTCD, shall take precedence over the requirements of this Section.

2.2 TRAFFIC CONTROL

The safe movement of traffic through construction areas depends upon communicating concise and proper information to the public by signs, barricades, delineators, flagmen and warning lights. All such devices necessary during construction shall be furnished by the Developer or the Contractor.

The size, shape and color of such devices as shown herein, shall be as required by the State of California, Department of Transportation.

2.3 SIGNS

The types of signs shown in Std. Detail 2-B are typical under normal conditions.

Warning signs used for nighttime conditions shall be reflectorized or illuminated. The use of orange flags in conjunction with signs is permitted if they do not at any time interfere with a clear view of the sign face.

Reflectorized signs fastened to barricades or similar supports shall have the face of the sign vertical and normal to the direction of traffic for effective visibility.

Signs are to be used only as long as necessary and then removed. During periods when the signs are temporarily unnecessary, they shall be removed or have their message covered.

2.4 BARRICADES

Barricades are intended to impose an obstacle or close off the normal flow of travel. Approved barricades are shown on Std. Detail 2-C.

Barricades shall not be used unless they are needed to separate the motorist from objects of greater hazard than the barricades themselves. Barricades should never be used primarily for delineation. The use of non-standard types of barricades, such as drums, buckets, sandbags, etc., is prohibited.

2.5 FLASHER SUPPORTS

Portable flasher supports shall be as required by the State Department of Transportation as shown on Std. Detail 2-C.

2.6 DELINEATORS

The function of delineators is to channelize traffic. They shall consist of post and paddle type markers or cylindrical or cone shaped objects, 18 to 48 inches in height, as shown in Std. Detail 2-D.

Delineators should be uniformly positioned laterally and longitudinally relative to the line of traffic and they must be maintained in an erect position.

Delineators for night use shall be reflectorized or illuminated to be visible from 500 feet under normal conditions.

When placed in close proximity to the edge of a traffic lane, delineators should be made of a material that will withstand impact without damage to them or the striking vehicle.

Consideration must also be given to the necessity for stability against knockdown from wind or from the wind wash of passing traffic.

2.7 FLAGGER

A flagger is one of the oldest and most basic means of controlling traffic. A flagger can observe changing conditions and transmit information to the motorist based on current conditions. The flagger also acts as a guard in advance of a work party by observing approaching traffic, and being prepared to warn the workmen.

A flagger should be used only when such discretionary capability is required, and not as a substitute for other warning signs and devices.

When a flagger is necessary, the flagger must convey a message, and the message must be timely and accurate. The flagger's effectiveness and the safety of the traffic and his fellow workmen depend upon the way the flagger works. Standard hand signals shall be used as shown in the California MUTCD.

2.8 WARNING LIGHTS

Warning lights shall be electric lanterns, electric markers or flashers provided to indicate an obstruction or restriction during periods of low visibility. Warning lights shall be placed to mark the location of obstructions. Motion may be imparted to warning lights.

Warning lights may be fastened to signs, barricades and portable flasher supports in a manner satisfactory to the Director.

2.9 STREET CLOSURE

During the period of construction, the Developer or Contractor shall maintain the public way so as to not unreasonably hinder, render inconvenient or interfere with the public use thereof and shall erect such barriers, signs and other measures as may be necessary and proper to provide for the safety of persons, animals and vehicles using the public right-of-way. The Director or designated representative shall be the sole judge of the interpretation of the provisions of this paragraph and all decisions rendered regarding same shall be final. When safety measures are required, all construction shall immediately cease until such measures are provided to the satisfaction of the Director or designated representative.

The complete closure of a street is allowed only when authorized by the City. If authorized, no closure shall occur until after a traffic detour plan has been prepared by the Contractor and approved by the Director. Such closure shall be accomplished only through the use of Type III Barricades as shown on Std. Detail 2-E.

Whenever possible one lane for each direction of through traffic must be maintained except where flagmen are provided to control traffic, then one lane may serve both directions. When trenching is necessary across intersecting streets, the work shall be done in such a manner as to maintain two-way traffic on cross streets at all times.

Where trench line crosses the entrance to private property, access to the property shall be maintained at all times by means of a suitable bridge, until the trench may be backfilled. Such bridges shall be properly guarded and illuminated at night. Where any crosswalk is cut by the trench, suitable bridging shall be constructed. Such bridging shall be at least 4 feet in width, shall have suitable hand railing, and shall be properly guarded and illuminated at night.

Permanent closures and temporary closures in new developments at dead end streets and where pavement narrows at the edge of the development shall be made as shown in Std. Detail 2-F.

SECTION 3

STREETS

3.1 GENERAL

All public streets shall be designed in accordance with acceptable engineering principles and shall conform to these Standards. Primary references for supplemental information and/or details include:

- Highway Design Manual, California Department of Transportation, Central Publication Distribution Unit, Sacramento, CA.
- Transportation and Traffic Engineering Handbook, Institute of Transportation Engineers, Prentice Hall, Inc., Englewood, N.J., 1982.
- Guidelines for Urban Major Street Design, Institute of Transportation Engineers, Washington, D.C.

3.2 GEOMETRIC DESIGN

The street widths shall conform with these Improvement Standards.

The gutter grade shall not be less than 0.30%. In special situations, the grade may be reduced if approved by the City Engineer.

The standard street cross slope shall be 2.5%. Where necessary to match existing facilities, the cross slope on new streets may vary from 1.5% to 5%. Replacement of existing travel lanes in order to meet cross slope criteria may be required..

The street shall be designed to collect storm water at intersections whenever possible.

Top of curb fall (in feet) around returns shall be per the following table:

<u>Radius</u>	<u>Retrofit Minimum</u>	<u>New Construction</u>	
		<u>Minimum</u>	<u>Maximum</u>
15	0.12	0.20	0.50
20	0.16	0.20	0.60
25	*	0.25	0.80
30	*	0.30	0.90
35	*	0.35	1.00
40	*	0.40	1.10
45	*	0.45	1.20
50	*	0.50	1.30
* Special Determination			

Horizontal curves shall have the following minimum radii:

Local and Cul-de-Sac Streets	-	300 feet
Collector Streets	-	500 feet
Commercial/Industrial Streets	-	650 feet
Arterial Streets	-	1,100 feet

There shall be a tangent between reversing curves of at least 150 feet on arterial and industrial streets and 50 feet on residential and collector streets.

Street intersections shall be as near right angles as practical. In no case shall the angle of an intersection be less than 70 degrees. Streets located on opposite sides of an intersecting street shall have a continuous centerline or have their centerline separated by not less than 125 feet. Greater distances may be required on higher classification streets based on anticipated turning volumes.

Where offset intersections are collector streets, commercial/industrial or arterial streets offset intersections shall be designed to avoid conflicting left turns.

Cul-de-sacs shall be constructed on all permanent dead-end streets. Temporary cul-de-sacs on dead-end streets that are to be extended may be required for turn-around depending on the length of the street.

The maximum length of a cul-de-sac shall be 500 feet as measured from the centerline of the intersecting street to the radius point of the cul-de-sac.

Cul-de-sacs shall be open end design to allow pedestrian access to adjacent streets unless otherwise approved by the Planning Director.

3.3 STRUCTURAL DESIGN

The R-value design method used by the California Department of Transportation shall be used as a basis to determine the structural section of the streets. Whenever the pavement calculations produce more than 8 inches of aggregate base, a safety factor may be used at the option of the Engineer.

The Traffic Index (T.I.) shall be determined from traffic counts where they are available, or as determined by the City Engineer. A 10-year design life shall be used.

Where sufficient information is not available to determine the structural section using the above data, the following minimums shall be used.

<u>Street</u>	<u>T.I.</u>
Arterial	9
Collector	7
Industrial	8
Local	6
Cul-de-sac	5

In no case shall the ultimate thickness of asphalt concrete be less than 3 1/2 inches for arterial streets and less than 2 1/2 inches for all other streets.

In no case shall the thickness of aggregate base be less than 6 inches for any street.

3.4 CLEARING

Clearing and grubbing shall be done in accordance with Sections 16-1.01, 16-1.02 and 16-1.03 of the State Standards.

3.5 EARTHWORK

Earthwork shall be performed as set forth in Sections 19-1 and 19-2 of the State Standards and in accordance with the recommendations of the geotechnical engineering investigation report prepared for the project.

All compaction of fill materials shall be in accordance with Section 19-5 of the State Standards; except that the Contractor or Developer will only be required to strip the original ground of vegetation and compact the top 6 inches of original ground in place to not less than 90% of maximum dry density in accordance with ASTM Designation D1557 before the fill is placed.

3.6 SUBGRADE

All clods shall be broken and all rocks, hard ribs and earth lumps over 2 1/2 inches in greatest dimension and other unsuitable material such as roots shall be removed from the job site. The top 6 inches of subgrade material shall be compacted to a firm, stable condition with approved equipment to not less than 90% of maximum dry density in accordance with ASTM Designation D1557.

The finished subgrade shall not vary more than 0.05 foot above the planned grade at any point. Care shall be taken to obtain compaction around existing manholes and water valves.

3.7 AGGREGATE BASE

The aggregate base material shall conform to the requirements of Section 26 of the State Standards for Class 2, 3/4-inch maximum combined grading.

An exception to Section 26-1.04 shall be that a single layer up to 0.7 foot may be permitted to be placed.

Motor graders may be permitted to spread and shape the aggregate base materials, provided this method does not result in segregation of material. The aggregate base shall be maintained in a well-mixed optimum moisture condition.

Contractor shall furnish the City a Certificate of Compliance for all aggregate base used.

3.8 ASPHALT CONCRETE

The asphalt concrete shall conform to the requirements of Section 39 of the State Standards. Asphalt concrete used in all but the final course shall be Type B with 3/4-inch maximum medium grading. Asphalt concrete used in the final course shall be Type B as follows:

- Arterial and Commercial/Industrial Streets - 3/4-inch maximum, medium grading
- Collector and Residential Streets - 1/2-inch maximum, medium grading

The asphalt grade shall be PG 64-10 as specified by the latest revision of the State Standards unless otherwise approved by the City Engineer.

Contractor shall furnish the City a Certificate of Compliance for all asphalt concrete used.

3.9 ASPHALT PAINT BINDER (tack coat)

An asphalt paint binder shall be applied in conformance with Section 39-4.02 of the State Standards.

3.10 HEADER BOARDS

Header boards shall be constructed to protect the edges of the asphalt concrete where streets are partially completed.

The boards shall be either Redwood or Douglas Fir with an American Wood Preservers Bureau Stamp indicating its use for ground contact and application of LP22 water borne preservative or approved equal.

The boards shall be 2-inch x 6-inch nominal dimension of appropriate material.

3.11 CONCRETE

Portland Cement concrete, unless otherwise specified, shall be Class 3 as defined in the State Standards with a minimum 28-day compressive strength of 2,500 pounds per square inch and a maximum slump of 3 inches. Concrete shall consist of Portland Cement, water and aggregate. Portland Cement shall be Type II. Aggregate shall be washed before use and be free from any foreign matter.

The aggregate shall be graded to provide a plastic, workable mixture of maximum size aggregate of 3/4-inches. The water shall be potable and no admixtures shall be used without approval of the Design Engineer and the City Engineer.

The cement, water and aggregates shall be combined at the batch plant and be thoroughly mixed. No water shall be added to the mixture after leaving the batch plant without approval of the Engineer. All concrete shall be in place within 1-1/2 hours from the time the cement is added to the aggregate.

The temperature of concrete as mixed and placed shall not be less than 55° Fahrenheit nor greater than 90° Fahrenheit.

The concrete shall be consolidated by tamping or vibrating. Concrete which has rock pockets or honeycombing after curing shall be removed and replaced.

All concrete shall be cured in accordance with Section 90-7 of the State Standards.

The Contractor shall furnish the City a Certificate of Compliance for all Portland Cement concrete used.

3.12 CURBS, GUTTERS AND SIDEWALKS

Curbs, gutters and sidewalks shall be constructed in accordance with these Improvement Standards and Section 73 of the State Standards.

Curbs, gutters and sidewalks shall not be placed monolithically. Where sidewalk is immediately adjacent to curbing, it shall be tied to the curb with No. 4 rebar at 4-foot on centers. Native material subbases under curbs, gutters and sidewalks shall be conditioned in accordance with the more stringent of the recommendations of the geotechnical report prepared for the project or the recommendations shown on Std. Details 3-E and 3F. Prior to placement of concrete, the Geotechnical Engineer shall provide the Director a written certification that the native material subbases were conditioned in accordance with these recommendations.

Sidewalks shall have a cross-slope toward the curb face of 1/8-inch per foot minimum to 1/4-inch per foot maximum.

Sidewalks shall be constructed utilizing fixed form construction in accordance with 73-1.05A of the State Standards.

Sidewalks which are required against the edge of right-of-way shall be placed 1 inch from the property line to provide space for lot corner monuments.

3.13 DRIVEWAY APPROACHES - GENERAL

Approaches shall be constructed in accordance with these Improvement Standards and Section 73 of the State Standards.

Commercial, industrial or other high volume driveway approaches on arterial and collector/industrial streets shall be located as far as practical from the nearest curb return and may be prohibited within 200 feet where the intersection is signalized or planned for signalization. All driveway approaches located on arterial and major collector streets, shall be a minimum of 50 feet from the end of the curb return.

No residential driveway approaches shall be located on arterial streets.

Driveway transitions shall clear all public facilities such as street light and traffic signal standards, utility poles, fire hydrants and street trees located behind sidewalks by a minimum of 5 feet. (If public facilities are located within parkways or sidewalk areas, this distance shall be increased to a minimum of 8 feet.

Driveways shall be designed to minimize vehicles scraping the pavement in front of the driveway or high-centering at the back of the driveway. For new work, residential driveway grades shall not exceed 8 percent when measured from the back of the sidewalk grade to the beginning of the building setback line. Changes in grade beyond the building setback line shall not exceed 12 percent within any 10-foot distance.

In designing overlays or driveway replacements, the deflection angle at the flowline determined by a point on the pavement 4 feet out from the flowline and at a point on the driveway 10 feet behind the flowline should not exceed 9.75 degrees.

All abandoned driveways shall be completely removed and replaced with standard curb, gutter and sidewalk. (Driveways are considered abandoned when on-site development, fencing or other uses demonstrate that the driveway is no longer needed.)

3.14 RESIDENTIAL DRIVEWAYS

For residential driveways the maximum width shall not exceed 28 feet. In addition, the maximum driveway approach width shall not exceed 50% of the lot frontage measured at the curb line or 34 feet whichever is less. For residential lots with more than one driveway approach, total combined driveway approach width shall not exceed those previously mentioned. The minimum width of the driveway shall be the driveway apron width; the maximum width of the driveway shall be the apron width plus 6 feet. Driveways shall align with the approach and shall not be less than the approach width for a distance of 20 feet as measured from the back of sidewalk.

A minimum of 6 feet of full height curb shall be maintained between the top of transitions of adjoining driveways. On cul-de-sacs and knuckle curves, this minimum width may be reduced to 2 feet.

3.15 COMMERCIAL DRIVEWAYS

Commercial driveways shall be used on all driveways serving four or more parking stalls and other locations where trucks, including garbage trucks are anticipated.

Special commercial driveways are those with a depressed back of driveway at property line and a radius instead of a curb transition is used to provide improved access at high volume driveways. These driveways shall be used when the driveway peak hour volume exceeds 300 vehicles per hour and may be used on lower volume driveways where allowed by the City Engineer.

A minimum of 20 feet of full height curb shall be maintained between the top of transitions or curb radius between adjacent driveways.

3.16 ALLEYS

Alleys shall be constructed in accordance with these Improvement Standards.

The subgrade, aggregate base and asphalt concrete shall be constructed as required in Sections 3.6, 3.7 and 3.8 respectively of these Improvement Standards.

3.17 VALLEY GUTTERS

Valley gutters designed to permit drainage across a street shall be constructed as shown on Std. Detail 3-H.

3.18 CURB RAMPS

Curb ramps shall be constructed at all intersections as shown on Std. Details 3-O or 3-P as applicable.

Curb ramps must comply with the latest requirements of Section 1127B.5 “Curb Ramps” of the California Building Code.

3.19 RAISING UTILITY BOXES

Utility boxes and manholes shall be raised by the Developer or Contractor to conform to these Improvement Standards. Utility boxes include, but are not limited to, sewer manholes, sewer cleanouts, water valves, storm drain manholes and survey monuments.

Where existing utility boxes are in the work area, their frames and covers shall be removed before subgrade compaction is made and a cover shall be placed to prevent dirt and loose materials from entering the facility.

Base and surface material shall be placed over the covers, after which the frames and covers shall be set to finish grade.

3.20 TESTING

Material testing to show conformance with these Improvement Standards shall be done in accordance with Section 1.8 of these Improvement Standards.

3.21 ACCESS CONTROL WALLS

Where required by the City, access control walls shall be constructed. Access control walls shall be reinforced masonry or concrete and shall be a minimum of 6 feet in height. Architectural approval of the wall design is required from the City Manager.

Walls shall be shown on the Improvement Plans. A building permit shall be obtained by the Developer from the Chief Building Official prior to commencement of work on the walls located outside public rights-of-way.

3.22 PAVING ADJACENT TO EXISTING STREETS

Whenever new curb and gutter is constructed along an existing street, paving of the adjacent roadway is required. Pavement structural sections shall be designed in accordance with the traffic index of the street.

If the structural section of the existing street along which curb and gutter is to be constructed is inadequate or substandard, the street shall be reconstructed to the centerline or brought to standard by calculated overlay thickness.

If the adjacent road conforms to current standards, the paving may extend to the existing edge of pavement.

SECTION 4

LIGHTING

4.1 GENERAL

The lighting system shall be designed to best serve the area and to minimize the length of service runs from the points of connection to the street lights.

All street lights to be installed shall be shown on the Improvement Plans. The location of the power source, all conduit runs and pull boxes shall also be shown on the plans. In addition, length of conduit runs and conductor sizes shall be shown on the plans. These items shall either be shown in tabular form or denoted next to each conduit run on the plans.

Spacing and locations for standard electroliers are shown on Standard Details 4-A and 4-B.

4.2 REGULATIONS AND CODES

Regulations and codes shall conform to Section 86-1.02 of the State Standards.

4.3 EXCAVATION AND BACKFILL

Excavation and backfill shall be in accordance with Section 86-2.01 of the State Standards.

4.4 CONDUIT

All conductors shall be installed in conduit except inside poles. Conduit shall be in accordance with applicable sections of Section 86-2.05 of the State Standards and shall be sized in accordance with the National Electrical Code; however, in no case shall the conduit be smaller than that indicated in Section 86-2.05b of the State Standards. Conduit shall be installed in accordance with the applicable sections of Section 86-2.05c of the State Standards. All risers shall be rigid steel.

4.5 PULL BOXES

Pull boxes shall be in accordance with Section 86-2.06 of the State Standards, except that all pull boxes shall be concrete. Pull boxes shall be located where shown on the plans or where required by regulations or codes. In addition, a pull box shall be installed immediately adjacent to the light base. Pull boxes shall be Christy N16 with N16J lids or equivalent if located outside of vehicular travel areas. Where located inside vehicular travel areas, lids shall be Christy N16-61J or equivalent. All pull box covers shall be marked "Street Lighting," and

installed at finished grade. Pull boxes shall be placed on a bed of pea gravel of 12-inch minimum thickness to allow drainage. Also, four concrete pavers or bricks shall be placed underneath the pull box to minimize settlement. Sufficient extensions shall be used to provide connection of conduits without offsets.

4.6 FOUNDATIONS

Foundations for poles shall conform to Standard Detail 4-D. Foundations shall be placed monolithically to within 4 inches of the sidewalk grade. After the pole is set, a square cap equal to the foundation diameter shall be installed to bring the foundation to sidewalk grade.

4.7 ANCHOR BOLTS

Anchor bolts shall be 1 inch in diameter, 36 inches long with a 4-inch "L" bend at the bottom end. All anchor bolts shall conform to ASTM Designation A-307.

4.8 CONDUCTORS

Conductors shall conform to Section 86-2.08 of the State Standards.

4.9 WIRING

Wiring shall be in accordance with Section 86-2.09 of the State Standards. In addition, the Contractor shall pull through a test mandrel to remove any foreign matter that may damage the insulation before pulling the conductor. No conductors shall be pulled until the conduit run is complete. Extreme care shall be exercised when pulling conductors and cable into conduits to avoid kinking, twisting, nicking or scratching of the conductors and insulation, or the placement of extreme stress on the conductors or cable.

A fused splice connector shall be used to connect the light to the line in accordance with Section 86-2.095 of the State Standard Specifications. However, the fused splice connector shall not be located in the pullbox but shall be located within the pole.

4.10 POLES

Standard electrolier poles shall be hot-dip galvanized steel. Thirty foot poles, shall be Ameron Catalog No. N-3015, Pacific Union Metal Manufacturing Company Catalog No. FS1130 P15 or an approved equal. Twenty-five foot poles shall be Ameron Catalog No. N-2512, Pacific Union Metal Manufacturing Company No. 71041-Y3-12 or an approved equal.

Poles shall not be installed until the foundation has set at least five (5) days. Poles shall be plumbed by adjusting the leveling nuts; leveling shims shall not be used.

Poles shall have hand-holes near their bases facing the street.

4.11 ARMS

Arm length for standard electrolier poles shall be as shown on Std. Detail 4-C.

4.12 ELECTROLIERS

Electroliers shall be located according to size as shown on Standard Details 4-A and 4-B. The luminaries shall be as follows or an approved equal:

200 watt	I.E.S. Type III	I.T.T. 113-62J3 G.E. M2RR20S1A2GMS2 Hubbell RM GT 15 S31 071 0 3 4 0
150 watt	I.E.S. Type II-4 or Type III	I.T.T. 113-56262 G.E. M2RR15S1M2GMS2 Hubbell RM GT 15 S31 061 0 3 4 0
100 watt	I.E.S. Type II or Type III	I.T.T. 113-56262 G.E. M2RR10S1M2GMS2 Hubbell RM GT 10 S31 061 0 3 4 0

Electroliers shall have high pressure sodium vapor lamps, glass refractors, built-in receptacles for photoelectric cells, and regulator or auto-regulator multi-tap (120/240 volt) ballasts with a power factor of not less than 92%.

4.13 PHOTOELECTRIC CELLS

Photoelectric cells shall be adjustable, compatible with related equipment and adequate for the load. They shall be General Electric C402G400 or an approved equal for standard electroliers.

4.14 TESTING

All streetlights shall be tested a minimum of 48 hours before acceptance by the City.

4.15 RECORD DRAWINGS

The Contractor shall provide the Design Engineer one (1) set of "Record Drawings" showing locations of all pull boxes, conduits, and street lights. The

Design Engineer shall include this information on the Record Drawings in accordance with Section 1.9 of these Improvement Standards.

SECTION 5

WATER

5.1 GENERAL

These Improvement Standards apply to all public water facilities designed for installation within a public right-of-way (ROW) or public utility easement (PUE) in the City and are limited to mains and services 12 inches or less in diameter. Standards and requirements for larger sizes will be determined by the City Engineer. However, all mains larger than 12 inches will be considered transmission mains, and no fire hydrants or water services will be connected to them.

In all developments, on-site mains and hydrants for fire protection shall be public. Other on-site facilities, unless specifically noted in these Improvement Standards or as required as part of project approval, shall be private and shall be designed and constructed in accordance with the provisions of the Uniform Plumbing Code, as adopted by the City.

Water lines shall be designed in accordance with acceptable engineering principles, California OSHA Standards, and California Department of Health Service Regulations, Title 22, Chapter 16, California Waterworks Standards, and shall conform to these Improvement Standards.

5.2 DESIGN SUBMITTALS

Prior to submittal of Improvement Plans for the first phase of construction, a master water plan for the entire development shall be submitted to the City Engineer for review and approval. The plan shall include flow and pressure calculations.

5.3 DESIGN FLOW

Unless other water usage figures are required or approved by the City, the following water demands shall be used for design calculations:

<u>Development</u>	<u>Average Daily Demand</u>	<u>Peaking Factors</u>	
		<u>Max. Day</u>	<u>Max. Hour</u>
Single Family Detached	500 gpud	2.7	4.05
Single Family Attached	500 gpud	2.7	4.05
Multi Family, 2-4 Units	400 gpud	2.7	4.05
Multi Family, 5 Units or More	350 gpud	2.7	4.05
Commercial, Industrial & Others	*	2.7	4.05

gpud = gallons per unit per day
* = To be determined on a case-by-case basis.

Design fire flows shall be as follows unless otherwise approved by the City Fire Chief:

<u>Land Use</u>	<u>Fire Flow</u>
Low Density Residential	1,500 gpm
Medium & High Density Residential	1,500 gpm
Commercial & Industrial	3,500 gpm

5.4 PIPE DESIGN

The minimum size pipe used for new water mains shall be 8 inches. Larger water lines may be required based on required fire flow, service demand or planned extension of the City's distribution system.

Transmission mains shall be any water line larger than 12 inches in diameter.

Permanent dead-end runs shall be no longer than 500 feet unless specifically approved by the City Engineer. Reasonable looping of water mains will be required.

The system shall be designed to maintain a minimum residual pressure of 20 psi at the service point or fire hydrant under the following demand conditions:

- maximum day flow plus fire flow, or;
- peak hour flow.

The Hazen-Williams formula shall be used to calculate design flow, pressure loss, velocity and pipe diameter relationships. The coefficient of friction, "C", shall be 120 for 10-inch and smaller lines and 130 for 12-inch and larger.

There are no slope requirements for water mains. However, inverts and pipe slopes shall be shown on the profile sheets of the Improvement Plans.

All high points within the system shall be located at fire hydrants or air release valves unless waived by the City Engineer.

The minimum cover over water mains shall be 3 feet in both paved and unpaved areas. The maximum cover over water lines should not exceed 5 feet.

When crossing a wastewater or storm line, it is desirable that the water main be installed above the other pipeline with a clearance of 12 inches. If the desired clearance cannot be maintained, the water main shall be designed in accordance with the requirements of the California Department of Health Services for separation between water mains and sanitary sewers as shown in Std. Detail 5-K.

Water mains shall be installed within street rights-of-way unless placement in an easement is specifically approved by the City Engineer. Alignment shall be parallel to the street centerline wherever possible.

Permanent easements shall be provided on all mains not located in public rights-of-way. The minimum easement width shall be 15 feet. The main shall be located in the center of the easement unless otherwise required by the Director.

Water mains shall not be placed in easements across low density or medium density residential lots.

A minimum horizontal clearance of 10 feet shall be maintained between water mains and wastewater or storm drain lines, unless otherwise approved by the City Engineer. If the 10-foot separation is waived by the City Engineer, the requirements of the California Department of Health Services for separation between water mains and sanitary sewers shall be adhered to.

Curved water mains are allowed; however, joint deflection or pipe curvature shall not exceed the pipe manufacturer's recommendations.

Valves on mains shall be spaced and located in conformance with the following criteria:

1. Water mains shall be valved on each side of railroad, highway and canal right-of-way crossings. (These valves shall be located outside of the right-of-way being crossed unless otherwise approved by the City Engineer.)
2. At "tees", valves are required as follows:
 - 2 valves where one leg is less than 10 inches in diameter, with one of the valves installed on the smaller leg.
 - 3 valves where all legs are 10 inches in diameter or larger.
3. At "crosses", valves are required as follows:
 - 3 valves where one or more legs is less than 10 inches in diameter with valves on each of the smaller legs.
 - 4 valves where all legs are 10 inches in diameter or larger.
4. At ends of mains or on stubs such that future extensions will not interrupt service.
5. At all fire hydrant assemblies.

The valving requirements mentioned above are minimum. Additional valves may be required.

Fire hydrants shall be spaced and located in accordance with the following criteria:

- Spacing shall not exceed 300 feet in residential areas along one side of the street. Hydrants shall normally be placed at street intersections.
- Spacing shall not exceed 300 feet in industrial areas.
- On streets classified as arterial or greater, the above spacing shall apply to both sides of the street. If buildings are separated from the street by a 6-foot or higher restrictive wall, hydrants shall be placed only at street intersections.
- At the ends of cul-de-sacs and permanent dead-end mains.
- At the end of all temporary dead end mains for blow-off purposes.

In commercial developments, locations and spacing shall be determined on a project-by-project basis by the City Fire Chief and City Engineer.

5.5 PIPE

The following standard pipe materials shall be used for water main construction and shall conform to the latest edition of appropriate American Water Works Association (AWWA) Standards:

<u>Pipe Materials</u>	<u>Standard</u>
Ductile Iron	AWWA C151
w/Cement Mortar Lining & Seal (std. thickness)	AWWA C104
Polyethylene Encasement	AWWA C105
PVC (iron pipe O.D.)	AWWA C900, DR 18 (Class 150) minimum

All water mains and services from the main to the meter shall be installed with 12 gauge TW single strand tracer wire.

5.6 FITTINGS

Bends, tees and other fittings shall be ductile iron conforming to AWWA C153.

Fittings shall be furnished with a joint type suitable for attaching to valves or other pressure pipe. Adaptor gaskets are not acceptable. Thrust blocking shall be

constructed at bends, tees, dead-ends and where changes in pipe diameter occur. Blocking shall be made of Class 3 concrete, and shall be placed between undisturbed ground and the fitting to be anchored. The area of bearing on the pipe and on the ground shall be that required by Std. Detail 5-I. The blocking shall be placed so that the joints of the pipe and fittings will be accessible for repair.

5.7 VALVES AND VALVE BOXES

Valves and valve boxes shall be installed at the locations shown on the plans.

Valves shall be Cast-Iron Resilient wedge Gate Valves conforming to AWWA C509 or Ductile-Iron Resilient Wedge Gate Valves Conforming to AWWA C515. Valves shall open left and be equipped with a 2-inch AWWA approved operating nut.

Valve boxes shall be Christy G5 with Christy Iron Cover or approved equals. All valve boxes shall be installed to finished grade as per Std. Detail 5-G.

5.8 SERVICES

Each parcel shall have individual water service. In general, only one service is to be provided per parcel served. Five exceptions are as follows:

- For residential developments with on-site public mains, one standard service per dwelling unit may be required.
- For individual commercial/industrial buildings with multiple suites, one standard service with a 1-inch water meter per suite shall be provided.
- In parcels with separate buildings, one standard service per building may be required (individual shut-offs per unit are required).
- In non-residential developments, separate water service(s) shall be provided for landscaped areas.
- In multi-family residential developments of 5 units or more, separate water service(s) shall be provided for landscape areas.

Due to the variety of building and main configurations and backflow requirements, the above requirements/exceptions are not specific. Details of each project shall be specifically approved by the City Engineer.

All water services shall be metered.

Size of service is to be determined by the Design Engineer for the parcel being served. (Minimum service size is 1-inch.)

A minimum horizontal clearance of 10 feet shall be maintained between water services and wastewater services, unless otherwise approved by the City Engineer.

For City maintained landscape areas, parks and detention basins, minimum water service sizes are as follows:

<u>Parcel Size</u>	<u>Waterservice</u>
Parcels less than 1.5 acres	2-inch
1.5 acres - 4.5 acres	3-inch
4.5 acres - 9.5 acres	4-inch
Parcels greater than 9.5 acres	To be determined

5.9 SAMPLING STATIONS

One sampling station shall be provided for every 40 water services or fraction thereof. Sampling stations shall be installed per Std. Detail 5-D.

5.10 TRENCH EXCAVATION

The Contractor shall, prior to beginning construction, obtain from the Division of Industrial Safety the permit required by California Labor Code, Section 6500, and pay any fee charged for such permit. In addition thereto, whenever the work under the Contract involves trench excavation 5 feet or more in depth, the Contractor shall submit for approval to a registered civil or structural engineer representing the City, in advance of excavation, a detailed plan showing the design of shoring, bracing, sloping, or other provisions to be made for worker protection from the hazard of caving ground during the excavation. If such plan varies from the shoring system standards established by the Construction Safety Orders of the Division of Industrial Safety, the plan shall be prepared by a registered civil or structural engineer. Nothing in this section shall be deemed to allow the use of shoring, sloping or other protective system less effective than that required by the Construction Safety Orders. Nothing in this section shall be construed to impose tort liability on the City, City Engineer, or any of their officers, agents or employees.

The pipe trench shall be dug with side walls sloped or otherwise supported in a safe manner in accordance with the Department of Industrial Relations, Division of Occupational Safety and Health Administration regulations pertaining to trenching.

Excavated material shall be placed on only one side of the trench unless otherwise directed. Separation distance between piles of excavated material and trench shall be consistent with the Construction Safety Orders.

The alignment and grade for the bottom of the trench shall be properly established before the trench is excavated and shall be approved by the City before the pipe is

laid. Trenches shall be true to line and grade, and the bottom shall be even and free from all objectionable material.

5.11 WATER IN TRENCH

When water is encountered in the trench, it shall be removed by draining or by pumping. Should water get into the trench before the pipe is laid, the laying of pipe shall be postponed until the trench has dried sufficiently to provide a firm foundation for the pipe or else, the mud or softer material shall be removed and grade re-established by backfilling and compacting with suitable material as determined by the City.

5.12 LAYING AND JOINTING OF PIPE

Laying and jointing of pipe shall be in accordance with the manufacturer's recommendations and as approved by the City.

Where rubber gaskets are used for jointing pipe, a feeler gauge shall be used to check the position of the rubber gasket upon each closure. The interior of the pipe shall be cleared of all debris, and exposed pipe ends shall be closed by a suitable pipe plug when pipe laying is not in progress.

The pipe shall be laid on a trench bottom shaped to provide adequate and uniform support of the pipe except at coupling or bell holes. The use of prepared mounds to facilitate laying of the pipe is not approved.

Where pipe is to be encased or have concrete bedding, suitable concrete blocks shall be used to support the pipe in the proper location while placing concrete.

5.13 HANDLING OF PIPE ACCESSORIES

Proper implements, tools, and facilities satisfactory to the City shall be provided and used by the Developer or Contractor for the safe and efficient execution of the work. All pipe, fittings, valves, hydrants and accessories shall be lowered into the trench in such a manner as to prevent damage to pipe fittings. Under no circumstances shall pipe or accessories be dropped or dumped into the trench. All foreign matter or dirt shall be removed from the interior of pipe before lowering into position in the trench. Pipe shall be kept clean by means approved by the Director during and after laying. All pipe and accessories shall be inspected for defects prior to lowering into trench. Any defective, damaged or unsound pipe or accessory shall be repaired or replaced at the Developer's or Contractor's expense.

5.14 SERVICE INSTALLATION

The services shall be installed per Std. Details. 5-A, 5-B and 5-C.

Special care shall be exercised to insure proper compaction is made under curb stop so it is vertical and the meter idler is level. Compaction shall be made under and around the meter box so it remains level and at the finished sidewalk grade.

5.15 FIRE HYDRANTS

Hydrants shall meet the requirements of AWWA C-502 for dry barrel hydrants with the drain outlet omitted and shall have two 2½-inch nozzles and one 4½-inch pumper nozzle. The main valve body shall have a diameter of at least 5¼ inches. The hydrants shall be a “traffic” type with a replaceable breakaway unit immediately above the groundline. The main hydrant valve shall close with water pressure when the hydrant is severed.

Hydrants shall be furnished with caps with rubber gaskets and chains. The contractor shall adjust the “bury length” of the hydrant to provide proper installation. The pumper nozzle shall be rotated to face the street after installation.

Hydrants shall be Waterous WB-67 or American Darling B84. All hydrants shall be furnished with a factory traffic yellow enamel finish coat.

5.16 CONNECTIONS WITH EXISTING WATER LINES

The Contractor shall make all excavations for connection to existing water lines and shall make connections to existing water line in the presence of the City.

Developer shall use an approved reduced pressure backflow preventer between the existing water main and the new line to fill lines of chlorination. City personnel must be the only ones to open or close existing water valves.

Connections shall be made at such times as designated by the City and in such a manner as to insure the least inconvenience to water users. No connection shall be made until the new work has been tested and disinfected as specified hereinafter. The Contractor shall be responsible for safeguarding the existing system from all damage and possible contamination in the performance of his work.

5.17 INSPECTION

1. All water lines shall be inspected for proper installation by the City, prior to backfilling of trenches.
2. HYDROSTATIC TEST

After installing pipe and prior to complete backfilling of trenches, the entire length of each line shall be subjected to a hydrostatic pressure of not less than 150 psi for a period of not less than 2 hours. Curb stops, idler fittings and fire hydrant units shall also be included in the hydrostatic test.

No pipe installation will be accepted if leakage for the section is greater than the allowable leakage determined by using one of the following formulas, as applicable:

$$\text{For Polyvinyl Chloride Pipe - } L = \frac{ND\sqrt{P}}{7400}$$

(L = Allowable leakage in gallons per hour. N = Number of joints in length of pipeline. D = Nominal diameter of the pipe in inches. P = Average test pressure in pounds per square inch.), or

$$\text{For Ductile Iron Pipe - } L = \frac{SD\sqrt{P}}{133,200}$$

(L = Allowable leakage in gallons per hour. S = Length of pipe tested in feet. D = Nominal diameter of the pipe in inches. P = Average test pressure in pounds per square inch.)

Water for testing may be taken from the nearest blow-off, fire hydrant or other approved source. All pipe, fittings, valves, couplings, and other materials needed to fill the test lines with water shall be supplied and installed by the Developer or Contractor. Care shall be taken not to contaminate the existing system.

The pump, gauge, pipe connection, and all necessary apparatus and equipment needed for the test shall be supplied by the Developer or Contractor.

The Developer or Contractor shall permanently stop all leaks. After repairing all defective items, the line shall be tested again to determine final acceptability of the installation.

3. DISINFECTING WATER MAINS

Water mains shall be disinfected in conformance with the procedure specified in the current standards for AWWA C651 "Disinfecting Water Mains" or other procedure satisfactory to the City Engineer. Only one connection shall be made between the existing City system and the system being disinfected unless otherwise approved by the Director. A temporary backflow prevention device as approved by the Director shall be installed between the two systems. Upon successful completion of the disinfection process, the temporary backfill prevention device shall be removed.

Chlorine shall be introduced into the lines at service connections or at such locations along the line that uniform distribution of chlorine throughout the line is ensured, including services, fire hydrants and stubs.

After final flushing, a testing laboratory, approved by the Director, shall take two consecutive sets of water samples from the system at least 24 hours apart. All samples shall be drawn from locations designated by and in the presence of the Director.

The system must pass both consecutive negative bacteriological tests prior to removal of the backflow prevention device.

Valves or other connections between the existing system and new system shall not be opened or completed until certification of the passing bacteriological tests have been received and reviewed by the Director.

5.18 BEDDING AND BACKFILL OF TRENCHES

After placement of bedding material, installation of the pipe and inspection, place sand backfill material simultaneously on both sides of the pipe in maximum 8-inch lifts keeping the level the same on both sides of the pipe. Place sand backfill to 6 inches minimum above the top of the pipe. Compact each lift to 90% of maximum dry density in accordance with ASTM D 1557.

Subsequent backfill material, to the bottom of the aggregate base, shall be imported material as approved by the City Engineer. Material shall be placed in 8-inch maximum lifts and each lift compacted to the relative density shown on Std. Detail 5-J.

Compaction tests shall be performed on each lift by a testing laboratory approved by the City. The testing laboratory shall be retained by the Developer and all testing expenses shall be paid by the Developer.

5.19 RESTORING SURFACE

The surface of all trenches shall be filled and compacted so that the surface will conform to the condition of the surrounding ground. The repaving requirements of the plans shall be met regardless of type of existing surfacing.

Existing pavement shall be cut in neat parallel lines as shown on Std. Detail 5-J.

Aggregate base shall be Class II compacted to 95% relative compaction. Aggregate base shall have 3/4-inch maximum combined grading.

Asphalt concrete shall be Type B, PG 64-10 with 1/2-inch maximum aggregate, medium grading.

A paint binder of asphaltic emulsion shall be applied to all surfaces in conformance with Section 39-4.02 of the State Standards.

SECTION 6

SEWER

6.1 GENERAL

These standards apply to all public sanitary sewer facilities designed for installation within a public right-of-way or public utility easement in the City. Except where specifically noted in these standards or as required as part of project approval, all sanitary sewer facilities installed on private property for private use and ownership shall be designed and constructed in accordance with the provisions of the Uniform Plumbing Code, as adopted by the City.

Sanitary sewer lines shall be designed in accordance with acceptable engineering principles, California OSHA Standards and State of California Title 22 requirements, and shall conform to City Standards. Storm water collection facilities shall not be connected to a sanitary sewer line.

6.2 DESIGN SUBMITTALS

Prior to submittal of Improvement Plans for the first stage of construction, a master sanitary sewer plan for the entire development shall be submitted to the City Engineer for review and approval. The plan shall include the following:

- A plan with a scale of 1" to 100' showing the proposed system, preliminary pipe sizes, tributary sub-areas and existing and future tributary areas outside the project area.
- Design flow at major junction points.
- A description and preliminary sketch of any pump stations. This information shall include number and size of pumps, wet well volumes and operating levels.

6.3 DESIGN

Unless specific sanitary sewer discharges are required or approved by the City, the following sewage discharges shall be used for design of residential developments:

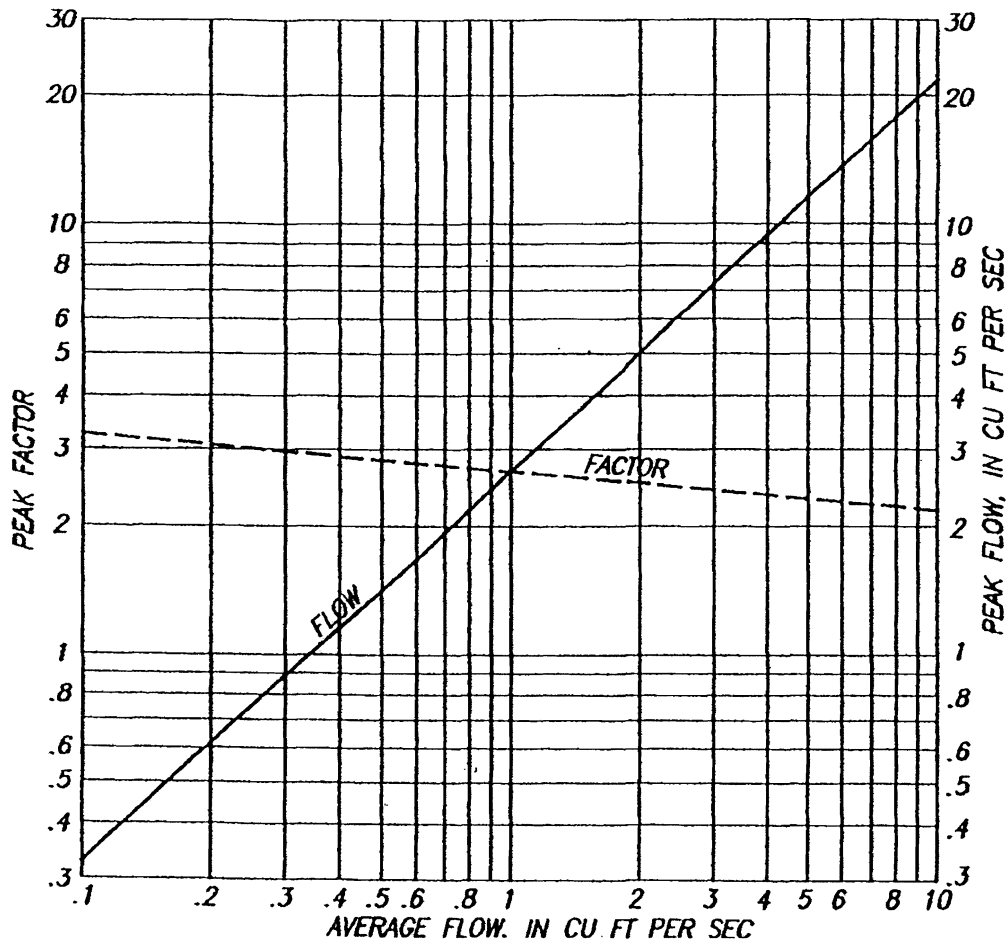
<u>Residential Development</u>	<u>Average Flow</u>
Single Family - Detached	300 gpud
Single Family - Attached	300 gpud
Multi Family - 2-4 Units	250 gpud
Multi Family - 5 Units or More	200 gpud

gpud = gallons per unit per day

For commercial, industrial and other types of development, anticipated sewage discharges shall be determined using typical discharge information from similar facilities.

Peak flow shall be obtained by multiplying the average flow by the peaking factor. The peaking factor is 3.0 for commercial flows and 2.0 for industrial flows.

Peaking factors for residential flow shall be selected from the following figure based upon total upstream average flow.



Sewer lines shall be sized to carry the peak design flow at less than 80% pipe capacity based on Manning's equation using an "n" value of 0.013.

Sanitary sewer calculations shall be submitted to the City Engineer for review.

The minimum size pipe used for gravity sanitary sewer mains shall be 6 inches in diameter.

Minimum slopes for sewer lines are as follows:

Diameter (inches)	Minimum Slope (ft/ft)
6	.0049
8	.0034
10	.0025
12	.0019
15	.0014
18	.0011
21	.0009
24	.0008

The above slopes are intended to provide velocities of not less than 2.0 feet per second when flowing half full based on Manning's equation utilizing an "n" value of 0.013.

If it is impractical to meet these velocity standards, the minimum slopes can be waived by the City Engineer.

Maximum velocity shall not exceed 10 feet per second. The slope of sewer lines between manholes shall be constant.

The minimum cover for sanitary sewer lines, unless otherwise approved by the City Engineer, shall be 3 feet 6 inches from the existing or planned final grade, whichever is lower, to the top of the sewer pipe. Where the preceding minimum cover cannot be provided, the City Engineer may approve with use of ductile iron pipe.

When crossing a water main, it is desirable that the sewer line be installed below the water main with a minimum clearance of 12 inches. If the desired clearance cannot be maintained, the sewer line shall be designed in accordance with the requirements of the California Department of Health Services for separation between water mains and sanitary sewers. A minimum vertical clearance of at least 3 inches shall be maintained between a sewer line and a storm drain.

At points of convergence of sewer pipes of the same size, the invert of the incoming pipe shall be a minimum of 0.1 feet higher than the outflowing pipe.

(This 0.1 foot of elevation difference does not apply for laying of pipe through a manhole.) Under no circumstances shall the crown of the incoming pipe be below the crown of the outflowing pipe.

Sanitary sewer pipes shall be placed within street rights-of-way unless placement in an easement is specifically approved by the City Engineer. Alignment shall be parallel to the street centerline whenever possible.

Permanent easements shall be provided for all mains not located in public rights-of-way. The minimum easement width shall be 15 feet. Wider easements may be required by the Director for any lines over 12 inches in width or with an invert elevation 5 feet or greater below ground line. The line shall be located in the center of the easement unless otherwise required by the Director.

A minimum horizontal clearance of 10 feet shall be maintained between sewer lines and water mains, unless otherwise approved by the City Engineer. If the 10 foot separation is waived, the requirements of the California State Department of Health Services for separation between water mains and sanitary sewers shall be adhered to.

6.4 PIPE FOR SEWER MAINS

The following standard pipe materials shall be used for gravity sewer construction and shall conform to the latest edition of American Society of Testing Materials standards (ASTM Standards).

<u>Pipe Material</u>	<u>Standard</u>
PVC (Std. Wall) Vitrified Clay	ASTM D3034 (SDR 26) ASTM C700

6.5 SERVICES

In general, only one sanitary service is allowed per parcel served. Additional services may be approved by the City Engineer in order to eliminate the need for on-site lift stations or monitoring requirements.

The minimum diameter for services (sewer laterals) shall be 4 inches.

A sewer lateral installed concurrently with a main sewer shall be of the same type and class of pipe material as the sewer main with the following exception:

- Where cover or water main separation require otherwise.

Sewer laterals shall not be connected to mains that are greater than 12 inches in diameter or to mains with a depth of cover greater than 10 feet.

6.6 MANHOLES

Manholes shall be placed at the intersections of all sewer mains and/or laterals 6 inches in diameter or larger and at sections where changes in slope, pipe size, or pipe alignment occur. In addition, terminal manholes shall be installed at the upstream ends of all mains. (Temporary cleanouts may be utilized at the upstream ends of mains intended for extension if approved by the City Engineer.)

Manholes shall have a maximum spacing of 350 feet on 6 inch and 8 inch mains and 450 feet on mains 10 inches and larger.

Manholes shall have an inside diameter of 4 feet when the largest pipe entering or exiting the manhole is less than 24 inches in diameter. Where the nominal pipe diameter is 24 inches or larger, the inside diameter of the manhole shall be 5 feet.

Drop manholes shall be provided wherever the invert of the incoming pipe is higher than 2 feet above the invert of the out flowing pipe.

All manholes on mains 15 inches and larger shall be polyurethane lined. The lining material shall be a two component polyurethane or polyurethane elastomer with primer. Material shall be Polibrid 670-5 primer with 705 topcoat as manufactured by Polibrid Coatings, Inc. or equal. Minimum dry film thickness shall be 80 mils. Lining shall be installed per manufactures recommendations.

6.7 LIFT STATIONS

Lift stations shall be specifically approved by the City Engineer. Lift stations shall utilize above ground pumps and shall be provided with fencing, paved access as required in Section 7.16 of these standards and potable water for cleaning purposes.

The following criteria shall be used for design:

- Wet well capacity shall be adequate to provide a minimum pump cycle time of 10 minutes.
- Wet wells shall be reinforced concrete with a polyvinyl chloride liner mechanically locked into the wall of the pipe.
- A stainless steel debris basket with a stainless steel slide rail system shall be provided on the influent pipe.

- An emergency overflow shall be provided between the wet well and the downstream collection system.
- Two non-clog centrifugal sewer pumps. Each pump shall be capable of pumping 100% of the design flowrate.
- A pole mounted yard light.

6.8 CONTROLS

Controls shall be mounted in a deadfront free standing self-contained NEMA 3R steel enclosure with a padlockable door. The control center and all electrical components shall bear the Underwriters Laboratory (UL) label.

An interior dead front aluminum door shall be provided with a continuous aircraft type hinge, shall contain cutouts for mounted equipment, and provide protection of personnel from live internal wiring. A breaker handle shall be provided on the main breaker and cutouts shall be provided on all other breakers to allow operation of breakers without entering the compartment. All control switches, indicator pilot lights, elapsed time meters, controller and other operational devices shall be mounted on the external surface of this door.

All circuit breakers shall be heavy-duty thermal magnetic or motor circuit protectors similar and equal to Square D type FAL. Each motor breaker shall be adequately sized to meet the pump motor operating characteristics and shall have a minimum interrupting capacity of 10,000 amps at 230 vac. Heavy-duty breakers shall individually control the control circuit and the duplex receptacle.

Circuit breakers shall be indicating type, providing "on-off-trip" positions of the operating handle. When the breaker is tripped automatically, the handle shall assume a middle position indicating "trip".

Thermal magnetic breakers shall be quick-make and quick-break on manual and automatic operation and have inverse time characteristics secured through the use of bimetallic tripping elements supplemented by a magnetic trip.

Breakers shall be designed so that an overload on one pole automatically trips and opens all legs.

A main circuit breaker sized for all pumps operating shall be installed in the enclosure.

Each pump shall be provided with the following:

- A motor starter that is NEMA rated sized for the pump horsepower per NEC. The overload heater shall be melting alloy type with protection

provided in each power phase. The starter coil shall be replaceable from the front without removal of the starter from its installed position. Overload heaters shall be sized for the full amp draw of the pump motor.

- A three position H-O-A switch. The switch shall be NEMA 4X rated with 10 amp contacts. A position indicating legend plate shall be provided.
- A green run pilot indicator light.
- A run time meter. The meter shall operate on 120 vac, shall indicate in hours (6 digits) and tenths and shall be non-resettable.

A three phase power monitor that will protect the system from over and under voltage conditions shall be installed. The unit shall provide protection for reversed phasing and loss of a phase in addition to the over voltage and under voltage conditions.

The control center shall also have a thermostatic controlled condensation heater and a 120 volt, 15 amp, GFI receptacle.

A UL listed manual generator power transfer switch with a receptacle and closing plug shall be provided to allow connection of an emergency power generator. For 3 phase 240 volt service, the receptacle shall be a Hubbel Model 4100 B9W. For 3 phase 480 volt service, the receptacle shall be a CROUSE HINDS Model AR 2042 S22 M80 receptacle.

The controller and water level sensor for pump control shall be as specified by the Director.

6.9 DISCHARGE PIPING

Discharge piping shall be ductile iron. Plastic piping may be allowed below ground, where approved by the City Engineer.

The design velocity in the discharge piping shall not exceed 8 feet per second. All internal piping in the pumping station should be properly anchored and restrained. Expansion joints and flanged connections shall be provided to facilitate dismantling and maintenance of the equipment.

Valving, couplings, and additional flanges as required for proper operation and maintenance of the pumping facilities shall be readily accessible.

6.10 FORCE MAINS

Force main piping shall be sized to provide a minimum velocity of 2 feet per second at the design flowrate of the lift station.

The following standard pipe materials shall be used for force main construction and shall conform to the latest edition of applicable AWWA Standards:

<u>Pipe Materials</u>	<u>Standard</u>
Ductile Iron	AWWA C151
W/Cement Mortar Lining & Seal (std thickness)	AWWA C104
Polyethylene Encasement	AWWA C105
PVC (iron pipe O.D.)	AWWA C900, DR 18 (Class 150) minimum

There are no slope requirements for force mains. However, inverts and pipe slopes shall be shown on the profile sheet of the Improvement Plans.

Force mains shall enter the gravity sewer system through a manhole. The invert of the force main shall be 1 foot above the flow line of the outflow pipe. Cleanouts or automatic air release valves as required by the City Engineer shall be provided.

6.11 TRENCH EXCAVATION

The Contractor shall, prior to beginning construction, obtain from the Division of Industrial Safety the permit required by California Labor Code, Section 6500, and pay any fee charged for such permit. In addition thereto, whenever the work under the Contract involves trench excavation 5 feet or more in depth, the Contractor shall submit for approval to a registered civil or structural engineer representing the City, in advance of excavation, a detailed plan showing the design of shoring, bracing, sloping, or other provisions to be made for worker protection from the hazard of caving ground during the excavation. If such plan varies from the shoring system standards established by the Construction Safety Orders of the Division of Industrial Safety, the plan shall be prepared by a registered civil or structural engineer. Nothing in this section shall be deemed to allow the use of shoring, sloping or other protective system less effective than that required by the Construction Safety Orders. Nothing in this section shall be construed to impose tort liability on the City, City Engineer, or any of their officers, agents or employees.

The pipe trench shall be dug with side walls sloped or otherwise supported in a safe manner in accordance with the Department of Industrial Relations, Division of Occupational Safety and Health Administration regulations pertaining to trenching.

Excavated material shall be placed on only one side of the trench unless otherwise directed. Separation distance between piles of excavated material and trench shall be consistent with the Construction Safety Orders.

The alignment and grade for the bottom of the trench shall be properly established before the trench is excavated and shall be approved by the City before the pipe is laid. Trenches shall be true to line and grade, and the bottom shall be even and free from all objectionable material.

6.12 WATER IN TRENCH

When water is encountered in the trench, it shall be removed by draining or by pumping. Should water get into the trench before the pipe is laid, the laying of pipe shall be postponed until the trench has dried sufficiently to provide a firm foundation for the pipe or else, the mud or softer material shall be removed and grade re-established by backfilling and compacting with suitable material as determined by the City.

6.13 LAYING AND JOINTING OF PIPE

Laying and jointing of pipe shall be in accordance with the manufacturer's recommendations and as approved by the City. Joint deflections shall not exceed 80% of the maximum recommendations of the manufacturer.

Where rubber gaskets are used for jointing pipe, a feeler gauge shall be used to check the position of the rubber gasket upon each closure. The interior of the pipe shall be cleared of all debris, and exposed pipe ends shall be closed by a suitable pipe plug when pipe laying is not in progress.

The pipe shall be laid on a trench bottom shaped to provide adequate support of the pipe except at coupling or bell holes. The use of prepared mounds to facilitate laying of the pipe is not approved.

Where pipe is to be encased or have concrete bedding, suitable concrete blocks shall be used to support the pipe in the proper location while placing concrete.

6.14 LATERAL CONNECTIONS

Where 4 inch laterals are constructed concurrently with main sewers, connections shall be made with a regularly manufactured wye branch. If approval is granted, a City approved wye or tee saddle shall be used. Saddles shall have a skirt to prevent the saddle from entering the sewer main beyond the inside surface of the main, and shall be located so the invert of the saddle branch is at the same elevation as the crown of the main sewer. A neat opening shall then be cut in the main sewer which shall form a snug fit with the spigot of the saddle. The saddle shall then be installed and secured in accordance with the manufacturer's recommendations and, in addition, shall be attached to the main with a minimum of two stainless steel banding straps. The ends of the lateral shall be securely stopped with plugs or caps which can easily be removed without damage to the pipe end. The end of the

lateral shall be marked by imprinting an "S" on the curb face over the sewer lateral. All laterals shall be installed prior to air testing.

Lateral connections to existing sewers shall be made at a manhole or wye branch.

Whenever cut-in wyes are utilized that require the usage of rubber couplings, the couplings shall utilize stainless steel shear rings to assist in maintaining proper alignment of the sewer

Where, in the opinion of the Director, it is impractical to connect or to install a wye branch, the connection shall be made by the use of a tee saddle as described above.

Any lateral larger than 4 inches in diameter shall be connected to the main at a manhole.

6.15 BACKFILL

After the sewers and appurtenances have been properly constructed and inspected, the trench shall be backfilled and compacted as shown on Std. Detail 6-H for flexible walled pipe and as shown on Std. Detail 6-I for rigid walled pipe.

Class I or sand backfill material, as applicable, shall be placed simultaneously on both sides of the pipe in maximum 8-inch lifts keeping the level the same on both sides of the pipe. Place backfill to 6 inches minimum above the top of the pipe. Compact each lift to 90% of maximum dry density in accordance with ASTM D 1557.

Subsequent backfill material, to the bottom of the aggregate base, shall be imported material as approved by the City Engineer. Material shall be placed in 8-inch maximum lifts and each lift compacted to the relative density shown on Std. Detail 6-H for flexible walled pipe and as shown on Std. Detail 6-I for rigid walled pipe.

Compaction tests shall be performed on each lift by a testing laboratory approved by the City. The testing laboratory shall be retained by the Developer and all testing expenses shall be paid by the Developer.

6.16 CLEANING AND FLUSHING

After all backfilling is completed and manhole frames and covers set, but prior to placement of paving material, the Contractor shall clean and flush all sanitary sewer mains.

6.17 INSPECTION

PVC sewer pipe shall be tested after cleaning and flushing using a mandrel or other approved testing device. Maximum deflection shall not exceed 5% of the average inside diameter of the pipe.

Any over-deflected pipe shall be re-laid.

Mandrels shall be rigid, nonadjustable, odd-numbering-leg (9 legs minimum), having an effective length not less than its nominal diameter. The minimum diameter of the mandrel at any point along its full length shall be as follows:

<u>Pipe Material</u>	<u>Nominal Size (Inches)</u>	<u>Minimum Mandrel Diameter (Inches)</u>
PVC – ASTM D3034 (SDR26)	6	5.33
	8	7.11
	10	8.87
	12	10.55
	15	12.90

Mandrels shall be fabricated from steel, fitted with pulling rings at each end, and stamped or engraved on some segment, other than the runner, with the pipe material, specifications, nominal size and mandrel O.D.

All sanitary sewer lines shall be pressure tested by a low-pressure air test. Each section of PVC pipe shall be tested in accordance with UNI-B-6. VCP shall be tested in accordance with ASTM C828. Any section of sewer pipe failing the air test shall be repaired and retested until leakage is reduced to acceptable leakage.

6.18 CLOSED CIRCUIT TV INSPECTION

Prior to placing the final street surfacing, the Contractor, will inspect all new sewer systems with a closed circuit television system. This will be done after the pipe has been installed true to the prescribed lines and grades, the trench backfilled and compacted, the manhole and cleanout covers set to proper grade, the roadway subgrade compacted, aggregate subbases and bases placed and compacted, and the sewer system cleaned of all debris.

At the start of each sanitary sewer section, the Contractor shall record the manhole location by street intersections the inspection is beginning and ending at. This information shall appear in typewritten letters on the videotape. A gauge shall be attached to and dragged behind the camera to indicate the depth of any standing water within the line. The gauge shall have a diameter of 10% of the pipe diameter being televised.

Pulling of the camera shall be stopped and locations recorded in typewritten letters on the video tape at the following locations:

- The beginning and ending locations of all areas where the depth of standing water exceeds 10% of the pipe diameter.
- All wye locations.
- Any problem areas.

Camera pulling speed shall not exceed 100 ft. per minute.

Copies of the videos shall be delivered to the Director for his review. The Contractor shall make all necessary repairs and corrections to the pipeline as required by the Director prior to paving.

6.19 GREASE INTERCEPTORS

Grease interceptors shall be constructed by the Developer on private property on the sewer service lateral for any facility whose operation will result in oil, grease, sand or other solids being discharged into the City's sanitary sewer system. (Interceptors or provisions to install future interceptors on all commercial/industrial shell buildings are also required.)

Interceptors shall conform to Section 708 and 711 of the Uniform Plumbing Code and Standard Detail 6-J. Interceptors shall be constructed outside the building where they can be easily inspected for proper operation by the City.

For additional information regarding specific requirements of grease interceptor, contact the Director.

SECTION 7

STORM DRAINAGE

7.1 GENERAL

All drainage design shall be in accordance with the following requirements and shall provide a positive means of drainage to the discharge point designated by the City. The use of retention basins or storage basins will be permitted only with specific prior approval by the City. All drainage calculations shall be submitted to the City Engineer for review. Drainage calculations shall cover all drainage facilities required to deliver run-off to a certain location and hydraulic grade line elevation as approved by the City Engineer. Drainage flow calculations shall be submitted on Std. Detail 7-A. Output of computerized calculations will not be accepted unless all of the data required on Std. Detail 7-A is provided in the same format as Std. Detail 7-A.

7.2 SUBMITTALS

Prior to submittal of Improvement Plans for the first phase of construction, a storm drainage master plan for the entire development shall be submitted to the City Engineer for review and approval. The plan shall include the following:

- A plan with a scale of 1" to 100' showing the proposed system, preliminary pipe sizes, tributary sub-areas and existing and future tributary areas outside the project area.
- Hydraulic calculations.
- Detention basin design calculations and conceptual drawings of the basin and access road. The drawings shall include approximate groundwater elevation, basin inverts, maximum water surface elevations and hydraulic grade line control elevations.
- A description and preliminary sketch of any pump stations or gravity outlet facilities. This information shall include number and size of pumps, sump volumes and pump operating levels.

7.3 FLOW RATES

Flow rates shall be determined by using the Rational Formula, $Q=CIA$. Where Q represents the quantity of run-off in cubic feet per second; A, the total run-off area in acres; I, the intensity of the rainfall in inches per hour as determined from the intensity duration curves shown on Std. Detail 7-B and the coefficient of run-off also shown on Std. Detail 7-B.

Roof to gutter time shall be assumed to be 20 minutes.

7.4 PIPE DESIGN

1. GENERAL

Storm drainage piping shall be designed to handle a storm with a minimum return period of ten years. The minimum size of any storm drainage pipe shall be 15 inches in diameter except for pipes terminated in cul-de-sacs and catch basin laterals which shall be a minimum of 12 inches in diameter.

Manning's formula shall be used to calculate design flow, velocity, slope and pipe diameter. Manning's roughness coefficient "**n**" varies with the type of pipe used according to the following table:

<u>Pipe Material</u>	<u>n</u>
Reinforced Concrete	0.013
Cast-In-Place Concrete	0.015

Reinforced concrete pipe shall be the minimum class required to serve the purpose intended but in no case shall be less than Class III conforming to the specifications for Reinforced Concrete Culvert, Storm Drain and Sewer Pipe, ASTM Designation C76. The pipe shall be manufactured using the packer head method or shall be centrifugally spun. The pipe shall utilize a Bureau of Reclamation Type R-4 bell and spigot. Use of elliptical reinforcement is not allowed.

Cast-in-place concrete pipe shall only be used if approved by the City Engineer. Cast-in-place concrete pipe shall conform to Section 63 of the State Standards and these Improvement Standards.

2. SLOPE

Minimum slopes are as follows:

Diameter (inches)	Minimum Slope (ft/ft)
12	0.0019
15	0.0014
18	0.0011
21	0.0009
24	0.0008
30	0.0006
36	0.0006
42	0.0005
48	0.0004

The above slopes are intended to provide velocities of not less than 2.0 feet per second when flowing half full regardless of the slope of the hydraulic grade line. (Where the City's system is surcharged, velocities based on the design hydraulic grade line are well below 2 feet per second except in trunk lines.)

If it is impractical to meet these velocity standards, the minimum slopes can be waived by the City Engineer.

The slope of storm drains between manholes shall be constant.

Catch basin laterals shall have a minimum fall of 0.10 feet between the catch basin and the manhole. Desired fall is 0.30 feet or more.

Siphons are not permitted.

3. VERTICAL ALIGNMENT

The minimum cover for storm drains shall be 2 feet 6-inches. When crossing a water main, the storm drain line should be installed below the water main with a minimum clearance of 12-inches. At points of convergence of pipes, the invert of the inflowing pipe shall be a minimum of 0.1 foot higher than the invert of the outflowing pipe. (This 0.1 foot of elevation difference does not apply for laying of pipe through a manhole.)

4. HORIZONTAL ALIGNMENT

Drainage pipes shall be placed within street rights-of-way unless placement in an easement is specifically approved by the City Engineer. Alignment shall be parallel to the street centerline whenever possible.

Permanent easements shall be provided for all mains not located in public rights-of-way. The minimum easement width shall be 15 feet. Wider

easements may be required by the Director for any lines over 18 inches in width or with an invert elevation 5 feet or greater below ground line. The line shall be located in the center of the easement unless otherwise required by the Director.

A minimum horizontal clearance of 10 feet shall be maintained between drain lines and water mains, unless otherwise approved by the City Engineer. If the 10-foot separation is waived, the requirements of the California State Department of Health Services for separation between water mains and sanitary sewers shall be adhered to.

7.5 DRAIN INLETS

Drain inlets shall be as shown on Std. Detail 7-J. The structural channel iron shall be galvanized to conform to the requirements Section 75-1.05 of the State Standards.

Spacing of drain inlets shall be such that the surface flow to the drain inlet does not encroach into vehicular travel ways.

7.6 MANHOLES

Manholes shall be located on storm trunk and lateral pipelines. Manholes shall be placed at all storm drain intersections, at sections where changes in slope, pipe size and alignment occur, and at the upstream ends of all storm drains.

Manholes shall have a maximum spacing of 500 feet.

Invert elevation drop across each manhole shall equal the difference in pipe diameter where there is a change in pipe size and a minimum of 0.1 foot at all bends.

7.7 TRENCH EXCAVATION

The Contractor shall, prior to beginning construction, obtain from the Division of Industrial Safety the permit required by California Labor Code, Section 6500, and pay any fee charged for such permit. In addition thereto, whenever the work under the Contract involves trench excavation 5 feet or more in depth, the Contractor shall submit for approval to a registered civil or structural engineer representing the City, in advance of excavation, a detailed plan showing the design of shoring, bracing, sloping, or other provisions to be made for worker protection from the hazard of caving ground during the excavation. If such plan varies from the shoring system standards established by the Construction Safety Orders of the Division of Industrial Safety, the plan shall be prepared by a registered civil or structural engineer. Nothing in this section shall be deemed to allow the use of shoring, sloping or other protective system less effective than that required by the

Construction Safety Orders. Nothing in this section shall be construed to impose tort liability on the City, City Engineer, or any of their officers, agents or employees.

The pipe trench shall be dug with side walls sloped or otherwise supported in a safe manner in accordance with the Department of Industrial Relations, Division of Occupational Safety and Health Administration regulations pertaining to trenching.

Excavated material shall be placed on only one side of the trench unless otherwise directed. Separation distance between piles of excavated material and trench shall be consistent with the Construction Safety Orders.

The alignment and grade for the bottom of the trench shall be properly established before the trench is excavated and shall be approved by the City before the pipe is laid. Trenches shall be true to line and grade, and the bottom shall be even and free from all objectionable material.

7.8 WATER IN TRENCH

When water is encountered in the trench, it shall be removed by draining or by pumping. Should water get into the trench before the pipe is laid, the laying of pipe shall be postponed until the trench has dried sufficiently to provide a firm foundation for the pipe or else, the mud or softer material shall be removed and grade re-established by backfilling and compacting with suitable material as determined by the City.

7.9 LAYING AND JOINTING OF PIPE

Laying and jointing of pipe shall be in accordance with the manufacturer's recommendations and as approved by the City. Joint deflections shall not exceed 80% of the maximum recommendations of the manufacturer.

Where rubber gaskets are used for jointing pipe, a feeler gauge shall be used to check the position of the rubber gasket upon each closure. The interior of the pipe shall be cleared of all debris, and exposed pipe ends shall be closed by a suitable pipe plug when pipe laying is not in progress.

The pipe shall be laid on a trench bottom shaped to provide adequate support of the pipe except at coupling or bell holes. The use of prepared mounds to facilitate laying of the pipe is not approved.

Where pipe is to be encased or have concrete bedding, suitable concrete blocks shall be used to support the pipe in the proper location while placing concrete.

7.10 BACKFILL

After the storm drains have been properly constructed and inspected, the trench shall be backfilled and compacted as shown in Std. Detail 7-C for rigid walled pipe and Std. Detail 7-D for cast-in-place pipe.

Sand backfill material, shall be placed simultaneously on both sides of the pipe in maximum 8-inch lifts keeping the level the same on both sides of the pipe. Place backfill to 6 inches minimum above the top of the pipe. Compact each lift to 90% of maximum dry density in accordance with ASTM D 1557.

Subsequent backfill material, to the bottom of the aggregate base, shall be imported material as approved by the City Engineer unless otherwise noted. Material shall be placed in 8-inch maximum lifts and each lift compacted to the relative density shown on Std. Detail 7-C for rigid walled pipe and Std. Detail 7-D for cast-in-place pipe.

Compaction tests shall be performed on each lift by a testing laboratory approved by the City. The testing laboratory shall be retained by the Developer and all testing expenses shall be paid by the Developer.

Jetting of backfill is not allowed.

During the compaction operation, the contractor must exercise extreme caution so as not to damage or disturb the pipe.

7.11 CLOSED CIRCUIT TV INSPECTION

Prior to placing the final street surfacing, the Contractor will inspect all new storm drain piping with a closed circuit television system. This will be done after the pipe has been installed true to the prescribed lines and grades, the trench backfilled and compacted, the manhole and cleanout covers set to proper grade, the roadway subgrade compacted, aggregate subbases and bases placed and compacted, and the sewer system cleaned of all debris.

At the start of each storm drain section, the Contractor shall record the manhole location by street intersections the inspection is beginning and ending at. This information shall appear in typewritten letters on the videotape. A gauge shall be attached to and dragged behind the camera to indicate the depth of any standing water within the line. The gauge shall have a diameter of 10% of the pipe diameter being televised.

Pulling of the camera shall be stopped and locations recorded in typewritten letters on the videotape at the following locations:

- The beginning and ending locations of all areas where the depth of standing water exceeds 10% of the pipe diameter.
- Any problem areas.

Camera pulling speed shall not exceed 100 ft. per minute.

Copies of the videos shall be delivered to the Director for his review. The Contractor shall make all necessary repairs and corrections to the pipeline as required by the Director prior to paving.

7.12 DETENTION BASINS

Detention basins shall be utilized only with prior approval of the City. Basins shall be either in-fill basins or dual use basins. In-fill basins shall only be used to serve lands located within the City limits for the year 2008.

The following minimum design criteria apply for all in-fill basins.

- Capacity shall be large enough to hold the total run-off from a 50-year frequency 24-hour duration storm.
- The maximum water surface of the basin shall be 1 foot below the elevation of the top of the curb at the lowest inlet within the tributary area.
- The maximum side slope shall be 3 horizontal:1 vertical.
- Basin bottom shall be provided with a minimum slope of 0.008 toward approved drainage facilities.
- Maximum depth of the basin shall be 10 feet.
- Storm discharge piping shall be designed such that nuisance flows less than the discharge capacity of the basin, shall be evacuated without first entering the basin.
- Basin shall be provided with outlet facilities capable of draining a full basin within 48 hours.
- The lowest point in the bottom of the basin shall be a minimum of 5 feet above the underlying groundwater table.
- Fencing as approved by the City shall be provided around the perimeter of the basin.

If the in-fill basin is to be maintained by the City, the following additional design criteria apply:

- A reinforced concrete inlet/outlet structure will be provided in the side slope of the basin.
- Paved access shall be provided from the public travelway to the inlet/outlet structure to allow cleaning during storm events.
- A maintenance access roadway with a minimum width of 14 feet shall be provided from the top of the basin to the bottom of the basin to allow access for maintenance vehicles.

Maintenance access roadways may be required from the adjacent travel way, to and around the basin. Maintenance access requirements shall be determined by the City Engineer on a site by site basis. Requirements for maintenance access may include, but is not limited to the following:

- A paved access roadway with a minimum width of 14 feet.
- Minimum outer and inner turning radii for maintenance vehicles of 42 feet and 24 feet respectively.
- A turn around area for maintenance vehicles to prevent them from having to back out onto public roadway.

For dual use basins, the criteria listed for in-fill basins apply in addition to the following:

- Capacity shall be large enough to hold the total run-off from a 100-year frequency 24-hour duration storm.
- The maximum side slope shall be 6 horizontal:1 vertical.

7.13 PUMP STATION

1. GENERAL

Pumping stations shall be designed to efficiently handle the calculated run-off from a storm with a ten-year return period unless utilized in conjunction with a detention basin. Pumps designed in conjunction with basins shall be capable of draining 100 percent of the basin's storage capacity within 48 hours.

Pump stations shall be designed with a separator to remove settling and floating debris from the water entering the pump sump. They will also be designed with the following criteria:

- Pump stations shall be duplex style stations with non-clog submersible FLYGT sewer pumps and slide rail systems or

non-clog low RPM vertical turbine or mixed flow pumps. Each pump shall be capable of pumping 100 percent of the design flow. If the design flow exceeds 2,000 gpm, a low flow pump shall be provided in addition to design rated pumps. The low flow pump shall have a capacity of 400 to 500 gpm and shall operate as the lead pump.

- Reinforced concrete pump sump of a hydraulic design that meets the recommendations of the pump manufacturer and the City.
- Minimum pump cycle time of 10 minutes.

2. SUBMERSIBLE PUMPS

Additional design criteria for stations utilizing submersible pumps is as follows:

- Stations comprised only of submersible pumps shall have aluminum access frames and covers with safety grate/cover such as the FLYGT Safe Hatch or Halliday Retro Grate provided on the wet well.
- Slide rails shall be 2-inch schedule 40 galvanized steel with stainless steel hardware.

3. CONTROLS

Controls shall conform to Section 6.8 of these standards.

4. DISCHARGE PIPING

Discharge piping shall conform to Section 6.9 of these standards.

7.14 TRASHRACKS

Trashracks shall be installed on all basin inlet/outlet structures and pump stations utilizing vertical turbine or mixed flow pumps.

Trashracks shall be constructed of flat steel bars a minimum of 2 inches deep and 0.25 inches wide. Centerline to centerline spacing of bars shall be 2 inches maximum.

Bars shall be held in a parallel, equally spaced position by a flat toe plate welded across their lower ends and by horizontal spacing bars welded to the rack's downstream side. These horizontal bars shall not interface with raking the racks.

Trashracks shall be inclined 30 to 45 degrees from the vertical and shall extend from the floor to the top of the structure. A walkway, platform or other suitable level surface shall be provided at the top of all structures to allow for proper maintenance operations. Guardrails meeting the Industrial Safety Orders shall be provided. Sufficient clearance shall be provided between trashracks and surrounding fences or other obstacles to permit handling of cleaning rakes.

7.15 ACCESS

Pump station layout shall allow for proper access of maintenance vehicles. Vehicular access route from the adjacent travelway throughout the site and back onto the public travelway shall be shown on the site plan. Access roads to pump stations shall be paved. Minimum outer and inner turning radii for maintenance vehicles of 42 feet and 24 feet respectively, are required. Minimum access road pavement width shall be 14 feet.

SECTION 8.1

PARKING

8.1.1 GENERAL

Parking lots shall be provided for commercial, industrial facilities and multi-family dwelling units containing more than four dwelling units located within the City. The minimum number of parking spaces for each use shall conform to the standards set forth in the Newman Municipal Code. All parking stalls shall be marked to clearly delineate the parking spaces.

End stalls shall be protected from the turning movements of other vehicles through the use of raised curbing and landscaping. Raised end islands or peninsulas shall extend a maximum of 15 feet past the parking base line to minimize pedestrian tripping.

All areas not utilized for parking and vehicular circulation shall be landscaped. Landscaped and paved areas shall be separated by 6-inch high concrete curbs.

8.1.2 PARKING DESIGN GUIDE

To aid in the design of parking layouts, the following information is offered as a guide to meet the minimum requirements for off-street parking and the layout design of driver-parking lots under normal use conditions.

The following factors should be considered:

- A. Sizes and Access: Each standard size off-street parking space shall have a minimum width of 9 feet and a minimum depth of 19 feet, exclusive of access drives or aisles, and shall be of useable shape, location, and condition. Compact spaces having a minimum width of 8 feet and a minimum depth of 16 feet, exclusive of access drives and aisles, shall be permitted, not to exceed 30% of the total required parking stalls. Small car spaces shall have “compact” or “small car” painted on the pavement at the entrance of each stall. End stalls and stalls adjacent to raised curbing shall be a minimum of 1 foot wider than the normal stall width. The length of the space may be reduced by 2 feet if landscaped planters of sufficient width are used as curb stops.
- B. Entrances, exits and location of nearest intersection, in each direction.
- C. The width of the parking area normally determines the parking angle to be used.

- D. Right angle (90 degrees) parking is usually more efficient and provides two-way movement in the aisles and shorter cruise distance. However, it generally requires more turning effort.
- E. Angle parking (other than 90 degrees) affords greater ease in parking and allows for narrower aisles but it requires one-way circulation.
- F. Delineating Striping: All parking spaces shall be clearly delineated using 4 inch wide striping. Length of striping shall be 17 feet for standard parking spaces and 14 feet for compact parking spaces. Length of striping may be reduced by 2 feet if landscaped planters of sufficient width are used as curb stops.

With these factors in mind, an accurate drawing of the proposed parking area should be prepared showing such details as sidewalks, curb cuts for driveways, use of abutting properties, immovable obstacles, flow of on-street traffic in the area, and other pertinent information. This drawing can be used to aid in the determination of a layout pattern based on selection of the best of all possible parking arrangements. The best arrangement should provide the maximum number of parking spaces with aisles and stalls designed for one-turn driver parking.

8.1.3 HANDICAP PARKING REQUIREMENTS

- A. Each parking area associated with any type of land use listed in the Newman Zoning Code, except for single-family and two-family residential dwellings, shall include a number of parking spaces specifically reserved for vehicles licensed or authorized by the State of California for use by physically challenged/disabled drivers in accordance with the following:

Total Spaces in Parking Area			Minimum Number of Spaces Required for Physically Challenged/Disabled Drivers
1	-	25	1 Van Accessible Space
26	-	50	2 including 1 Van Accessible Space
51	-	75	3 including 1 Van Accessible Space
76	-	100	4 including 1 Van Accessible Space
101	-	150	5 including 1 Van Accessible Space
151	-	200	6 including 1 Van Accessible Space
201	-	300	7 including 1 Van Accessible Space
301	-	400	8 including 2 Van Accessible Space
401	-	500	9 including 2 Van Accessible Space
501	-	1000	2% of total (1 of every 8 handicap parking stalls must be Van Accessible)

If the number of parking spaces exceeds 1,000, the minimum number of spaces provided for physically challenged/disabled drivers shall be 20, plus one for each 100 spaces over 1,000 or fraction over 1,001.

- B. Such parking spaces shall be located within a reasonable proximity of any conveniently accessible entrance to the building served by the parking area.

8.1.4 PAVEMENT

All parking stalls and drive aisles shall be paved with either asphalt concrete or Portland cement concrete.

A. Asphalt Concrete

Minimum slope for asphalt concrete shall be 1.5 percent. This slope requirement may be reduced to 1 percent by the City Engineer if it can be shown that meeting the 1.5 percent requirement is not practical.

B. Portland Cement Concrete

Unreinforced and reinforced Portland cement concrete pavement shall be designed and constructed in accordance with the recommendations of the American Concrete Institute set forth in ACI 330R-87, "Guide For Design And Construction Of Concrete Parking Lots." Design calculations meeting the criteria set forth in the above ACI guide shall be submitted to the City Engineer for review and approval.

Minimum finished grade slope shall be 1.0 percent. If it can be shown that meeting the 1.0 percent requirement is not practical, the slope may be reduced to 0.75 percent with the approval of the City Engineer.

8.1.5 STORM DRAINAGE

Design of storm drainage piping for parking lots shall be based on predicted run-off from a 5 year storm using the Rational Formula $Q = CIA$. Where Q equals the predicted run-off in cubic feet per second; C the coefficient of run-off; I the intensity of the rainfall in inches per hour; and A the drainage area tributary to the parking lot in acres. Run-off coefficients and intensity of rainfall shall be determined from Std. Detail 7-B.

Roof to parking lot time shall be assumed to be 10 minutes. Manning's equation shall be used for pipe flow design. When using smooth walled plastic pipe, the roughness coefficient, "n" value, shall be 0.010. Local entrance and exit losses

shall be included in the calculation of head loss. Local loss coefficients (K) shall be as follows:

LOCAL LOSS COEFFICIENTS	
<u>Local Loss</u>	<u>K</u>
Re-entrant Inlets	1.0
Square Edge and Beveled Inlets	0.5
All Exits	1.0
90° Elbow	0.3
45° Elbow	0.2
Line Flow Tee	0.2
Branch Flow Tee	1.0

If curb drains are used, all grate elevations shall be a minimum of 0.6 feet above the invert of the discharge pipe at the curb (exit).

If drain basin lines discharge into the City's storm drain system at the back of a City catch basin, the grate elevations in the parking lot shall be a minimum of 0.3 feet above the City's catch basin grate.

SECTION 8.2

CONCRETE WASHOUT

8.2.1 GENERAL

Federal law presently requires operators of construction sites one acre or larger (including smaller sites that are part of a larger common plan of development) to obtain authorization to discharge stormwater under an NPDES construction stormwater permit. As part of the permit, best management practices (BMPs) for concrete washout and disposal are required.

Any construction sites that are exempt from obtaining an NPDES construction permit and where concrete placement is occurring shall have a concrete washout as shown in Std. Detail 8-C. (The size of the concrete washout shall be adjusted according to project needs.)

Concrete washouts shall be constructed aboveground. Excess concrete shall be placed in the washout area until moisture has evaporated then concrete shall be broken up and disposed of properly.

SECTION 8.3

MONUMENTS

8.3.1 GENERAL

All survey monuments shall be visibly marked or tagged with the certificate number of the surveyor or civil engineer setting them, according to Section 8772 of the Professional Land Surveyors Act.

Permanent survey monuments set in pavement areas shall conform to Std. Detail 3-Q. Monuments found in a perishable condition shall be rehabilitated with a permanent monument according to Section 8773.3 of the Professional Land Surveyors Act.

The instructions as set forth in the 1973 "Manual of Instructions for the Survey of Public Lands of the United States", published by the Bureau of Land Management, Department of Interior, Washington, D.C., shall be followed for the restoration of lost or obliterated corners, for those corners that were established based on the Public Lands Surveying System.

Survey monuments shall be set at all lot corners, angle points and points of curvature. Monuments shall be 1-inch O.D. (3/4-inch I.D.) galvanized iron pipe, 24 inches in length set 6 inches below grade.

Subdivision boundary monuments, except those set in street pavement, shall be 1-1/2 inches O.D. (1-1/4-inches I.D.) galvanized iron pipe, 24 inches in length, set 12 inches below finish grade. Survey boundary monuments in street pavement shall conform to Std. Detail 3-Q.

Permanent survey monuments shall be set at all street centerline intersections, angle points and points of curvature. They shall also be placed at all section and quarter corners within the subdivision.

Section and quarter corner monuments shall be constructed and marked according to the 1973 "Manual of Instruction". Monuments located within street sections shall utilize a monument box conforming to Std. Detail 3-Q.

**IMPROVEMENT STANDARDS
STANDARD DETAILS**

TABLE OF CONTENTS

TRAFFIC CONTROL

2A	STREET NAME SIGN
2B	TYPICAL CONSTRUCTION SIGNS
2C	PORTABLE CONSTRUCTION BARRICADES
2D	TRAFFIC DELINEATORS
2E	STREET CLOSURE
2F	DEAD END STREET BARRICADE
2G	STOP SIGN INSTALLATION

STREETS

3A	ARTERIAL STREET SECTIONS
3B	COLLECTOR AND LOCAL STREET SECTIONS
3C	WALK THROUGH CUL-DE-SAC
3D	NON WALK THROUGH CUL-DE-SAC
3E	STANDARD CURB AND GUTTER SECTION
3F	SIDEWALK, CURB AND GUTTER SECTION
3G	SIDEWALK CONSTRUCTION
3H	VALLEY CROSS GUTTER
3I	RESIDENTIAL DRIVEWAY - ATTACHED SIDEWALK
3J	RESIDENTIAL DRIVEWAY - SEPARATED SIDEWALK
3K	COMMERCIAL/INDUSTRIAL DRIVEWAY
3L	SPECIAL COMMERCIAL DRIVEWAY
3M	ALLEY APPROACH DETAILS
3N	ALLEY GUTTER AND PAVEMENT SECTION
3O	CURB RAMP – TYPE 1
3P	CURB RAMP – TYPE 3
3Q	SURVEY MONUMENT AND WELL
3R	TREE PLANTING

LIGHTING

4A	ELECTROLIER LOCATIONS
4B	ELECTROLIER LOCATIONS
4C	ELECTROLIER INSTALLATION
4D	ELECTROLIER FOUNDATION DETAIL

WATER

- 5A 1" DIAMETER WATER SERVICE
- 5B 2" DIAMETER WATER SERVICE
- 5C WATER METER SERVICE DETAIL
- 5D SAMPLING STATION
- 5E DOUBLE CHECK VALVE BACKFLOW PREVENTER
- 5F REDUCED PRESSURE BACKFLOW PREVENTER
- 5G VALVE BOX DETAIL
- 5H FIRE HYDRANT INSTALLATION
- 5I THRUST BLOCKS DETAIL
- 5J WATER LINE TRENCH DETAIL
- 5K CROSSINGS

SEWER

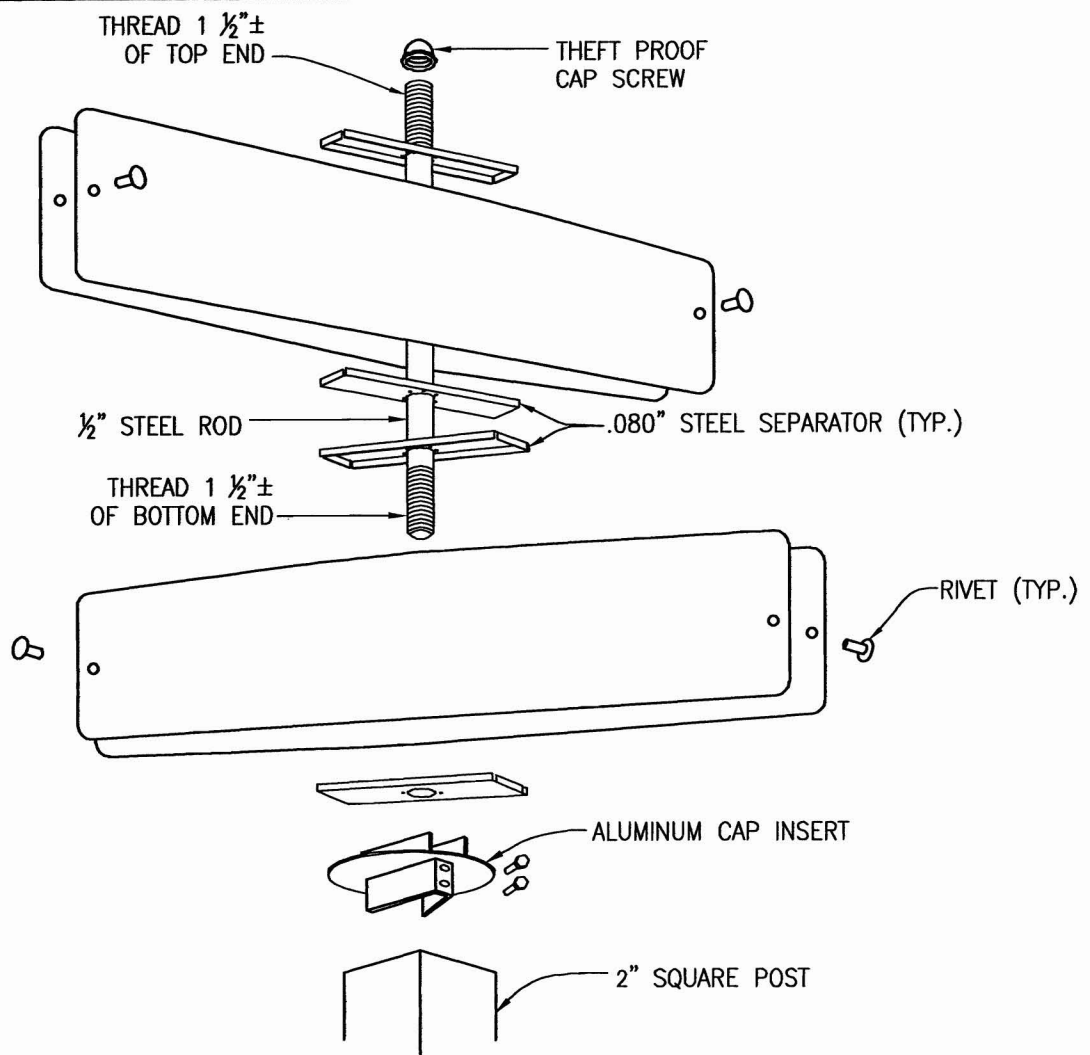
- 6A SANITARY SEWER MANHOLE
- 6B SANITARY SEWER DROP MANHOLE
- 6C TERMINAL MANHOLE
- 6D PAMREX MANHOLE FRAME AND COVER
- 6E SANITARY SEWER CLEANOUT
- 6F 4-INCH SEWER SERVICE
- 6G 4-INCH SERVICE CLEANOUT
- 6H FLEXIBLE WALL PIPE BACKFILL
- 6I RIGID WALL PIPE BACKFILL
- 6J GREASE & OIL INTERCEPTOR

STORM DRAINAGE

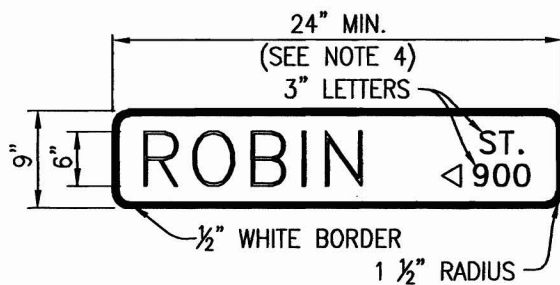
- 7A STORM DESIGN SHEET
- 7B RAIN INTENSITY FREQUENCY CURVES
- 7C RIGID WALL PIPE BACKFILL
- 7D C.I.P.P. STORM DRAIN AND TRENCH
- 7E STORM DRAIN MANHOLE FOR 30" – 48" C.I.P.P.
- 7F STORM DRAIN MANHOLE FOR 54" – 96" C.I.P.P.
- 7G STORM DRAIN MANHOLE PLAN VIEW (C.I.P.P.)
- 7H STORM DRAIN MANHOLE
- 7I PAMREX MANHOLE FRAME AND COVER
- 7J TYPE "A" CATCH BASIN

MISCELLANEOUS

- 8A OFF STREET PARKING STANDARDS
- 8B TYPICAL PARKING STALL STRIPING
- 8C CONCRETE WASHOUT – SMALL OPERATIONS



STREET NAME SIGN HARDWARE



STREET SIGN

NOTES:

1. STREET NAME SIGN TO BE PLACED 7 FEET, 6 INCHES ABOVE GROUND.
2. SEE STD. DETAIL 2-G FOR MOUNTING LOCATIONS AND INSTALLATION DETAILS.
3. STREET NAME SIGN SHALL BE 0.080-INCH ALUMINUM. BACKGROUND SHALL BE ENGINEERING GRADE 3M REFLECTIVE VINYL SHEETING. SIGN PLATE COLOR SHALL BE GREEN WITH A 1/2 INCH REFLECTIVE WHITE BORDER. SIGN LEGEND SHALL BE 3M REFLECTIVE WHITE VINYL "C" HIGHWAY GOTHIC LETTERING.
4. LENGTH OF 30 INCH AND 36 INCH MAY BE USED TO ACCOMMODATE LONGER STREET NAMES.
5. INDIVIDUAL STREET SIGN LAYOUTS SHALL BE APPROVED BY THE PLANNING DEPARTMENT PRIOR TO FABRICATION.

CITY OF NEWMAN IMPROVEMENT STANDARDS



STREET NAME SIGN

APPROVED BY:

G. R. Regan DATE: 7/14/09

STD. DETAIL

2 - A

ROAD
CLOSED

R11-2
48"x30"

DETOUR
AHEAD

ONE LANE
ROAD
AHEAD

W20-2
48"x48"

W20-4
48"x48"

FLAGGER
AHEAD

ROAD
WORK
AHEAD

ROAD
CLOSED
AHEAD

C9A (CA)
48"x48"

W20-1
48"x48"

W20-3
48"x48"

OPEN
TRENCH

C27 (CA)
24"x24"

END
ROAD WORK

G20-2
60"x24"

25
MPH

W6
24"x24"

DETOUR

M4-10
48"x18"

END
DETOUR

M4-8a
30"x18"

CITY OF NEWMAN IMPROVEMENT STANDARDS



TYPICAL CONSTRUCTION SIGNS

APPROVED BY:

Eric R. Guel

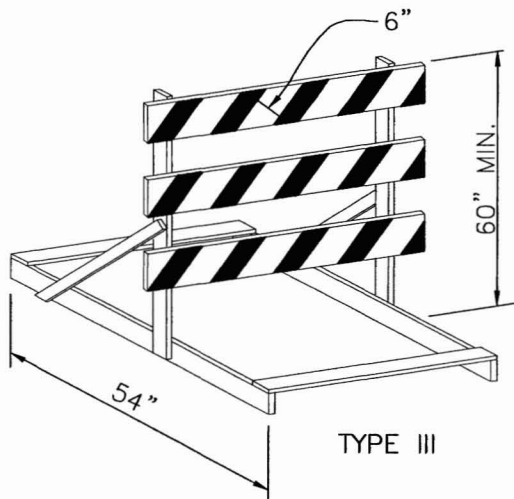
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STD. DETAIL

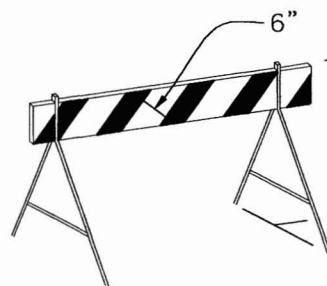
2 - B

BARRICADE AND PORTABLE FLASHER SUPPORT CHARACTERISTICS

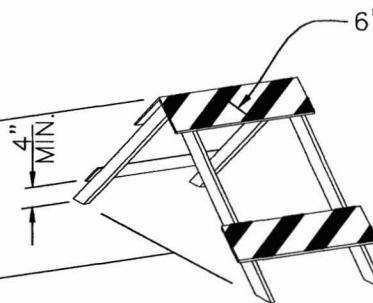
TYPE	1	2	3	PORTABLE FLASHER SUPPORT
WIDTH OF RAIL	8" - 12"	8" - 12"	8" - 12"	TOP - 8" - 12" BOTTOM - 4" MIN.
LENGTH OF RAIL	2' MIN.	32" - 4'	3' MIN.	31" MAX.
WIDTH OF STRIPE	6"	6"	6"	4" OR 6"
HEIGHT	3' MIN.	3' MIN.	5' MIN.	30" MIN.
NUMBER OF RAIL FACES REFLECTORIZED	2	4	3 IF FACING TRAFFIC IN ONE DIRECTION. 6 IF FACING TRAFFIC IN TWO DIRECTIONS.	2 IF TOP RAIL IS 12" WIDE. 4 IF TOP RAIL IS LESS THAN 12" WIDE



TYPE III



TYPE I



TYPE II

NOTES:

1. REFLECTORS AS REQUIRED BY THE CITY ENGINEER OR DIRECTOR OF PUBLIC WORKS.

CITY OF NEWMAN IMPROVEMENT STANDARDS



PORTABLE CONSTRUCTION BARRICADES

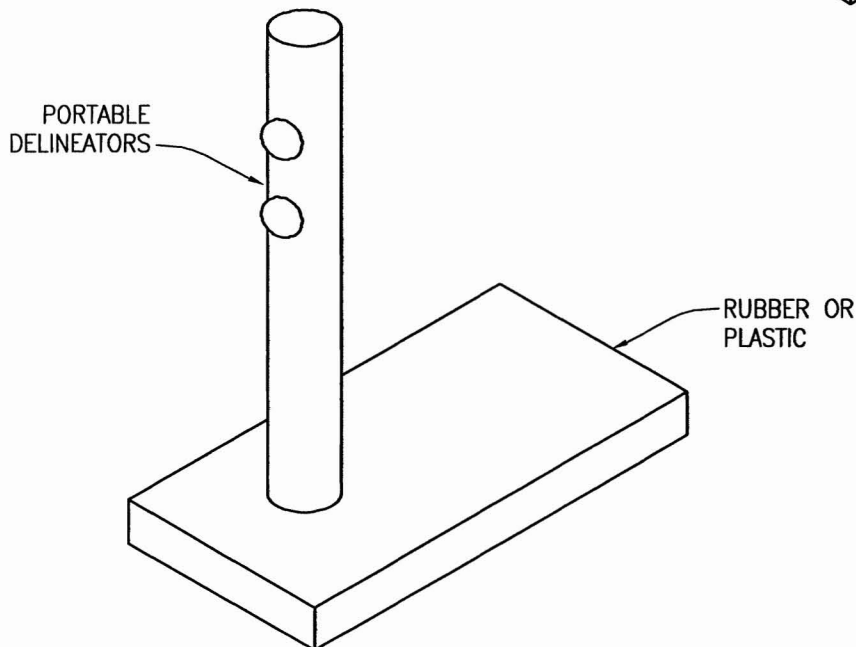
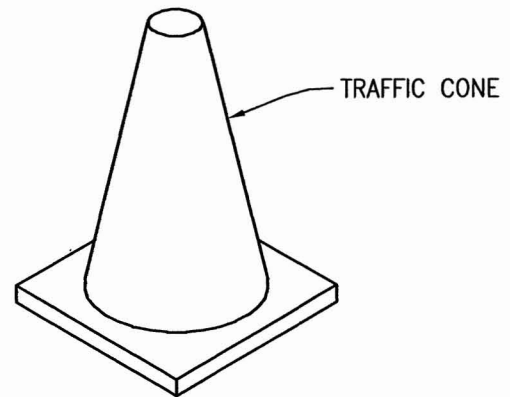
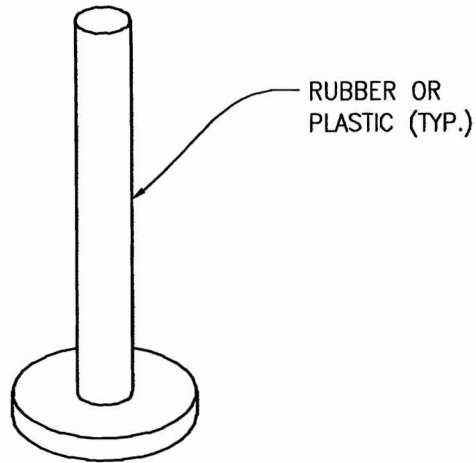
APPROVED BY:

G. R. Gue

DATE: 7/14/09

STD. DETAIL

2 - C



Jul 08, 2009 - 2:37pm
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CITY OF NEWMAN IMPROVEMENT STANDARDS

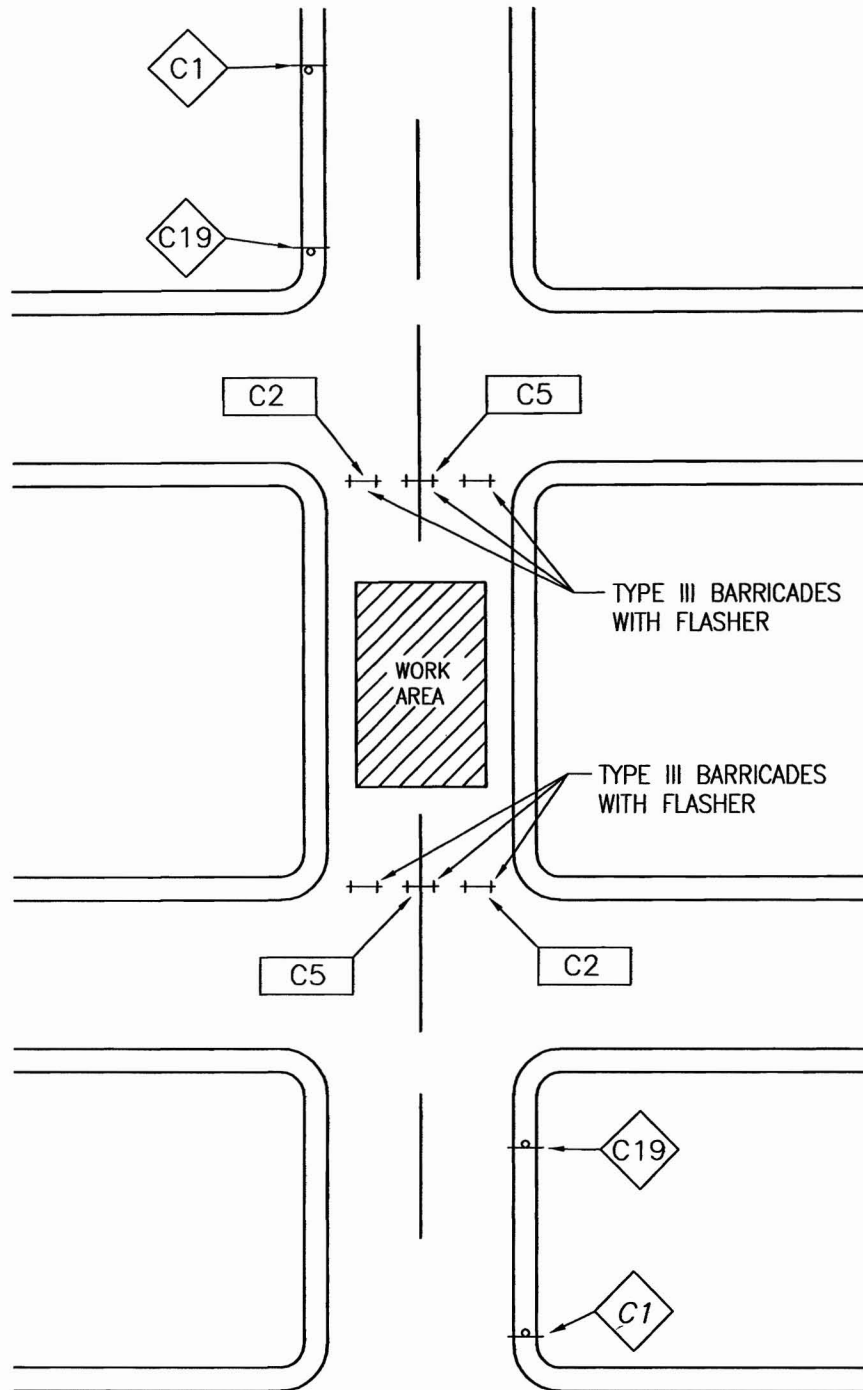


TRAFFIC DELINEATORS

APPROVED BY: *Era R. G. [Signature]* DATE: 7/14/09

STD. DETAIL

2 - D



NOTES:

1. PLACEMENT OF BARRICADES SHALL NOT PRECLUDE ENTRANCE OF EMERGENCY VEHICLES.
2. ADDITIONAL DETOUR SIGNS MAY BE REQUIRED BY THE PUBLIC WORKS DIRECTOR.

 OR  SIGN PER STD. DETAIL 2 - B

 TYPE III BARRICADE PER STD. DETAIL 2 - C

CITY OF NEWMAN IMPROVEMENT STANDARDS

STREET CLOSURE



APPROVED BY:

Eric R. Gural

DATE: 7/14/09

STD. DETAIL

2 - E



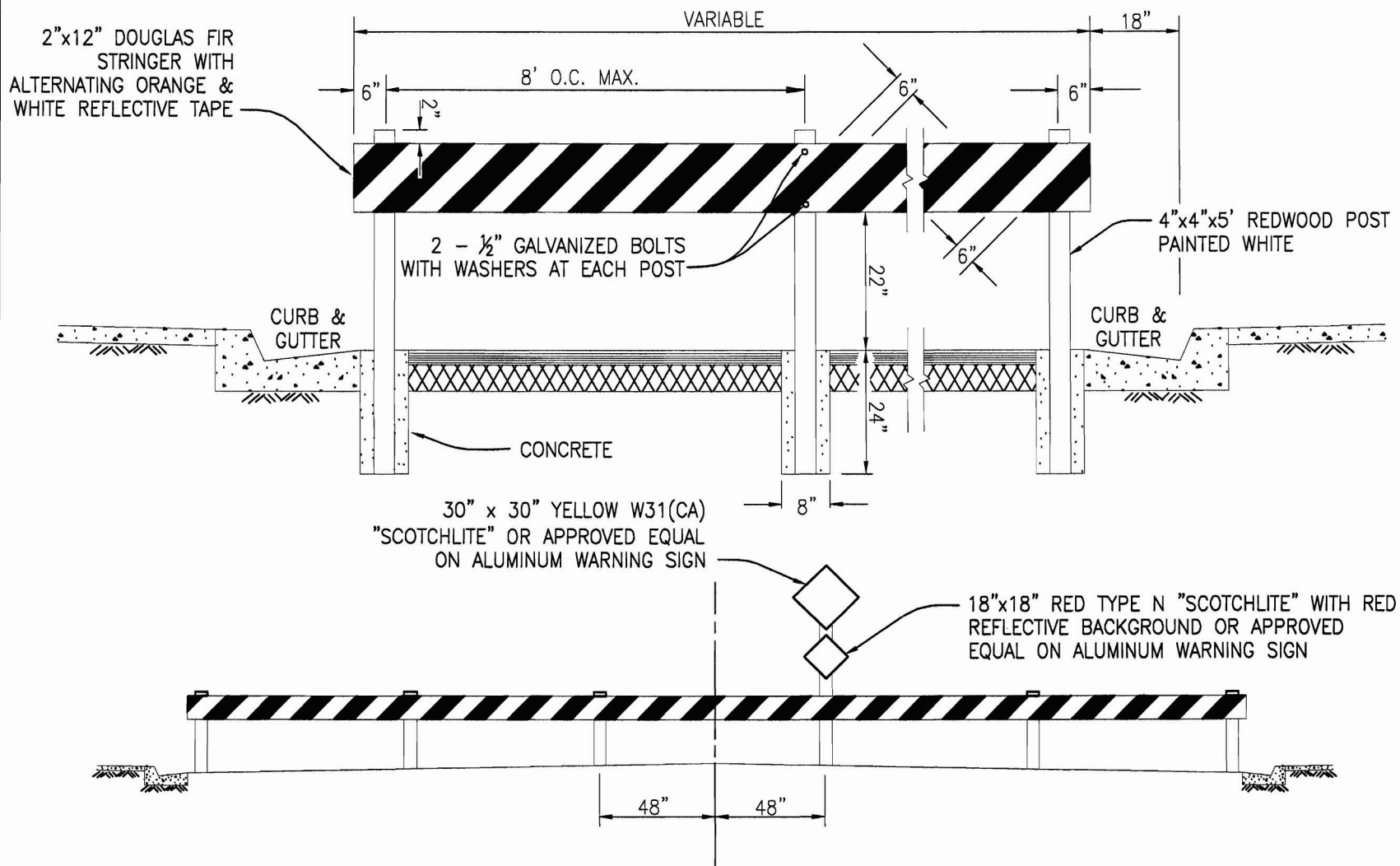
CITY OF NEWMAN IMPROVEMENT STANDARDS

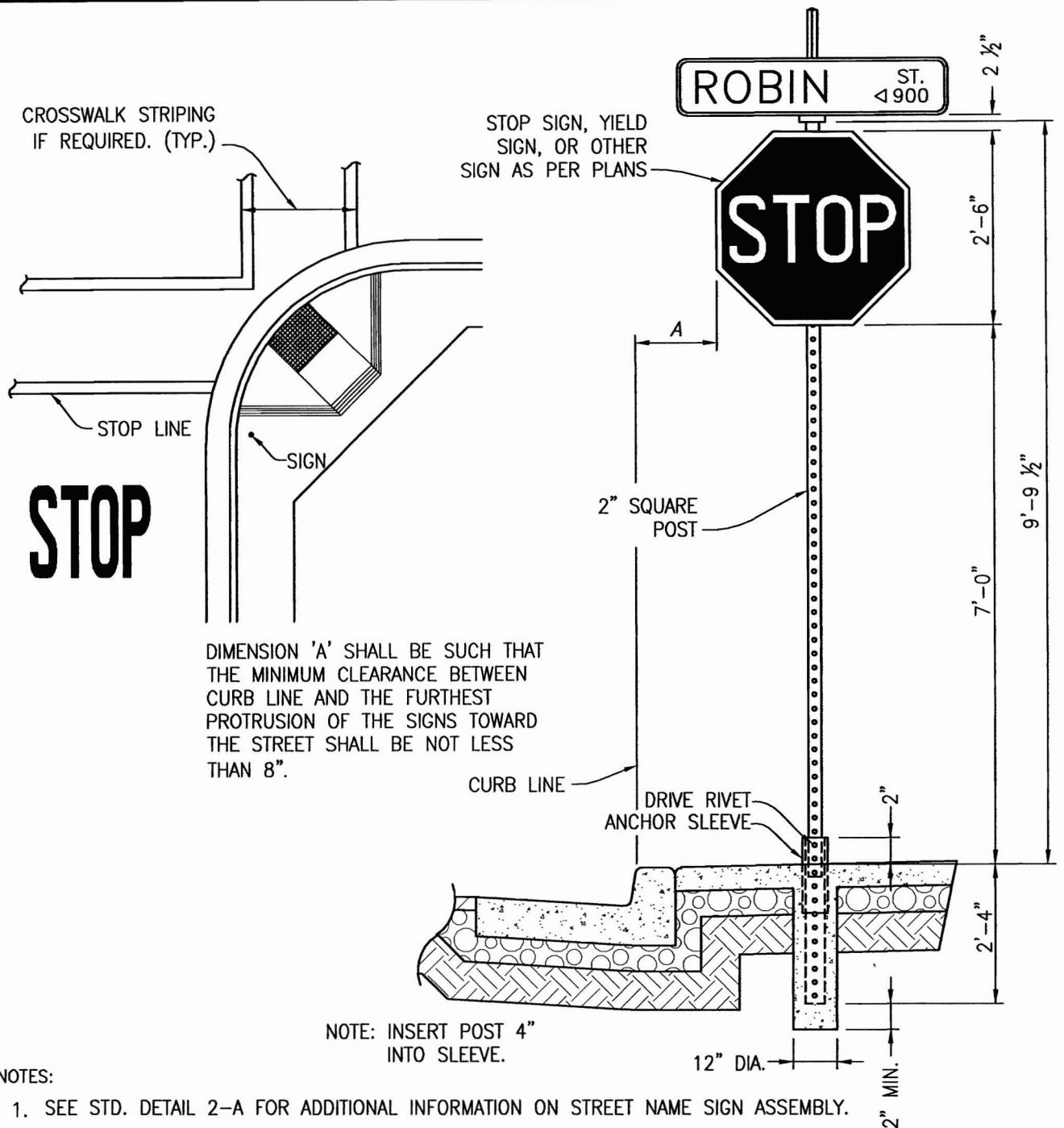
DEAD END STREET BARRICADE

APPROVED BY: *Garland* DATE: 7/14/09

STD. DETAIL

2 - F





NOTES:

1. SEE STD. DETAIL 2-A FOR ADDITIONAL INFORMATION ON STREET NAME SIGN ASSEMBLY.
2. CONCRETE FOOTING NOT REQUIRED IF POST INSTALLED IN CONCRETE.
3. STOP SIGNS SHALL BE REFLECTIVE TYPE: 3M ENGINEER GRADE REFLECTIVE MATERIAL OR EQUAL.
4. "STOP", PAVEMENT MARKING STOP LINE AND CROSS WALK STRIPING SHALL BE OF THERMOPLASTIC MATERIAL IN ACCORDANCE WITH SECTION 84-2, OF THE STATE STANDARD SPECIFICATIONS.
5. LOCATE POLE IN CONCRETE AREA UNLESS SHOWN OTHERWISE ON PLANS.
6. SIGN POST SHALL BE 14 GAUGE 2-INCH SQUARE PERFORATED GALVANIZED STEEL WITH 6 GAUGE 2 1/4-INCH SQUARE PERFORATED GALVANIZED STEEL ANCHOR SLEEVE AS MANUFACTURED BY UNISTRUT CORPORATION.

CITY OF NEWMAN IMPROVEMENT STANDARDS

STOP SIGN INSTALLATION



APPROVED BY:

Gar R. Ryd

DATE: 7/14/09

STD. DETAIL

2 - G



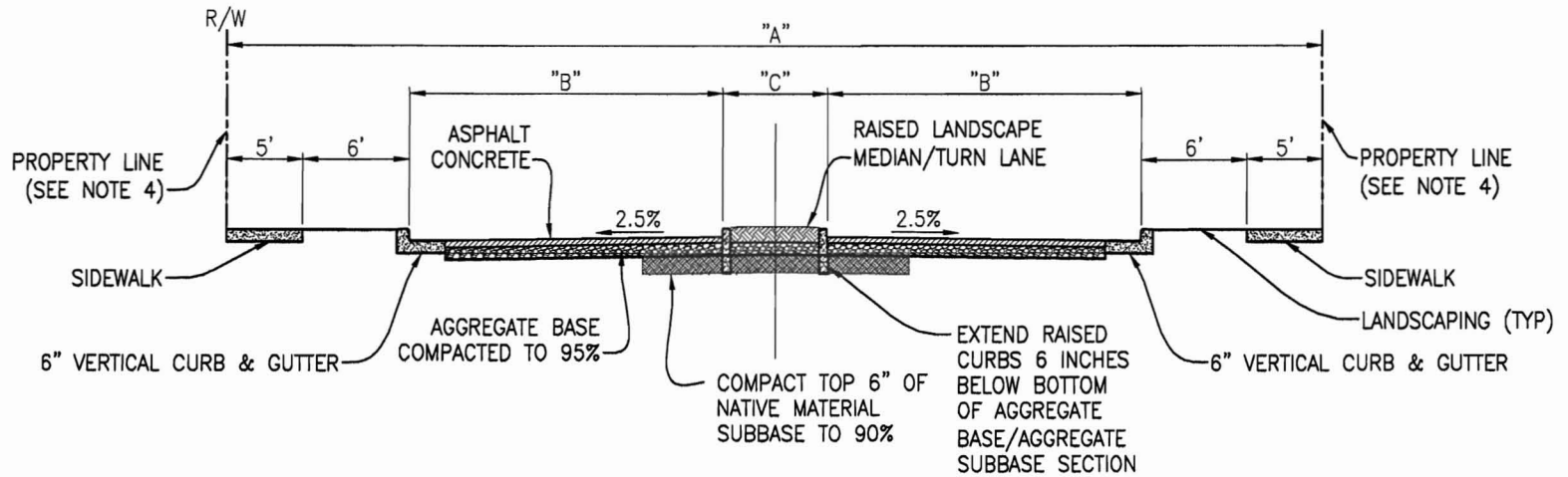
CITY OF NEWMAN IMPROVEMENT STANDARDS

ARTERIAL STREET SECTIONS

APPROVED BY: *[Signature]* DATE: 7/14/09

STD. DETAIL

3 - A



TYPICAL SECTION

STREET CLASSIFICATION	"A"	"B"	"C"
4-LANE ARTERIAL (PARKING AND BIKE LANES)	109'-0"	36'-0"	15'-0"
4-LANE ARTERIAL (NO PARKING AND 6-FOOT BIKE LANES)	96'-0"	30'-0"	14'-0"
2-LANE ARTERIAL (PARKING AND BIKE LANES)	70'-0"	24'-0"	0
2-LANE ARTERIAL (NO PARKING AND 6-FOOT BIKE LANES)	58'-0"	18'-0"	0

NOTES:

1. SOIL DENSITIES ARE EXPRESSED AS A PERCENTAGE OF MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM DESIGNATION D1557.
2. APPLY FOG SEAL TO PAVEMENT AFTER PAVING.
3. DUE TO SPECIFIC DESIGN CONSIDERATIONS THE ABOVE RIGHTS-OF-WAY AND CURB TO CURB WIDTHS MAY BE REVISED.
4. IF WALLS/FENCES ARE TO BE INSTALLED ALONG ARTERIALS, THE R/W WIDTH SHALL BE INCREASED TO PROVIDE LANDSCAPE BUFFERS BETWEEN THE BACK OF SIDEWALKS AND WALLS/FENCES.



CITY OF NEWMAN IMPROVEMENT STANDARDS

COLLECTOR AND LOCAL STREET SECTIONS

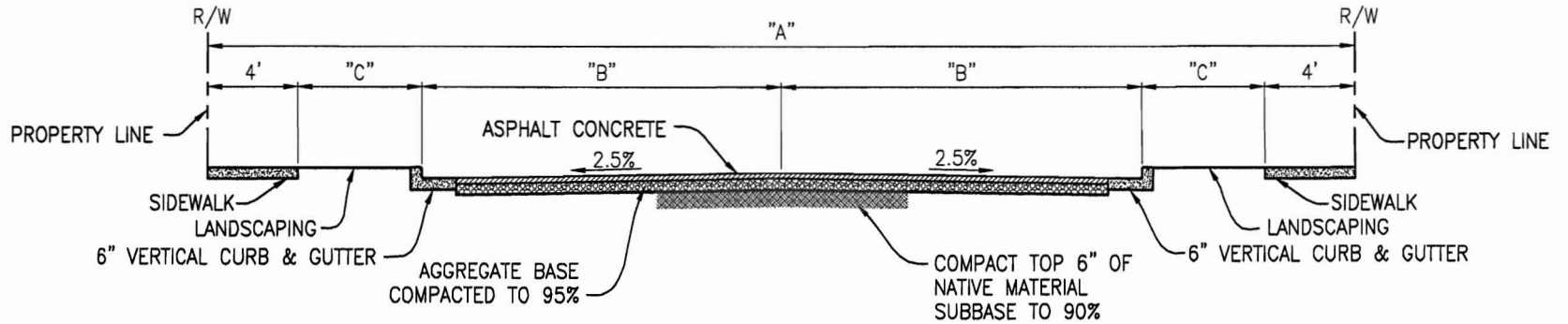
APPROVED BY:

[Signature]

DATE: 7/14/09

STD. DETAIL

3 - B



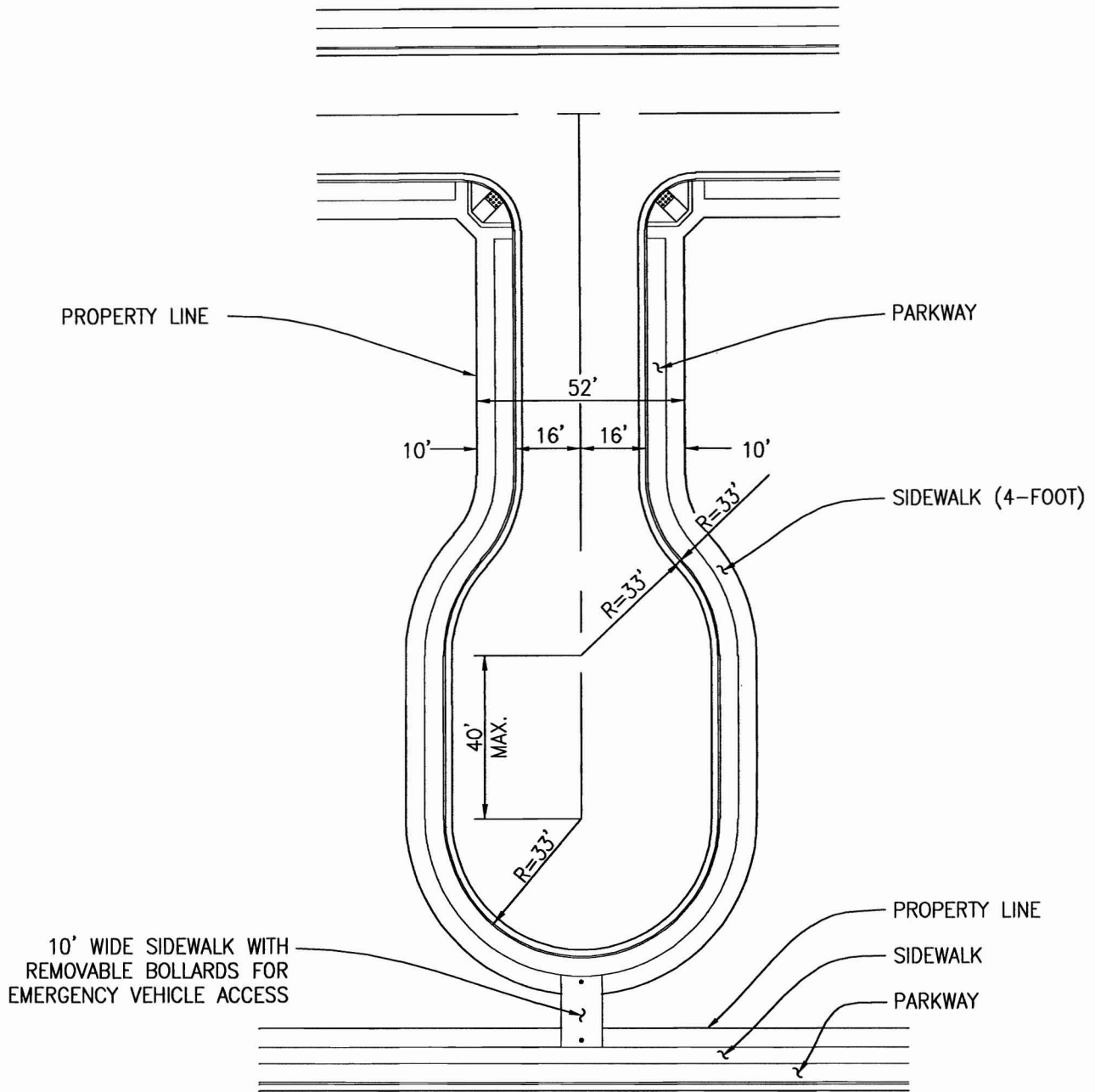
TYPICAL SECTION

STREET CLASSIFICATION	"A" IN FEET	"B" IN FEET	"C" IN FEET
MAJOR COLLECTORS (PARKING AND BIKE LANES)	68'-0"	24'-0"	6'-0"
MINOR COLLECTORS	60'-0"	20'-0"	6'-0"
LOCAL	58'-0"	18'-0"	7'-0"

NOTES:

1. SOIL DENSITIES ARE EXPRESSED AS A PERCENTAGE OF MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM DESIGNATION D1557.
2. APPLY FOG SEAL TO PAVEMENT AFTER PAVING.
3. DUE TO SPECIFIC DESIGN CONSIDERATIONS THE ABOVE RIGHTS-OF-WAY AND CURB TO CURB WIDTHS MAY BE REVISED.

SEE CITY OF NEWMAN STD. 3-D
FOR ADDITIONAL INFORMATION.



NOTES:

1. RADII SHOWN ARE FOR RESIDENTIAL CUL-DE-SACS. FOR COMERCIAL OR INDUSTRIAL CUL-DE-SACS RADII SHALL BE APPROVED BY THE ENGINEER.
2. LOT LINES FOR THE CUL-DE-SAC SHOWN SHALL BE PERPENDICULAR TO THE CENTERLINE OF THE CUL-DE-SAC.

CITY OF NEWMAN IMPROVEMENT STANDARDS



WALK THROUGH CUL-DE-SAC

APPROVED BY:

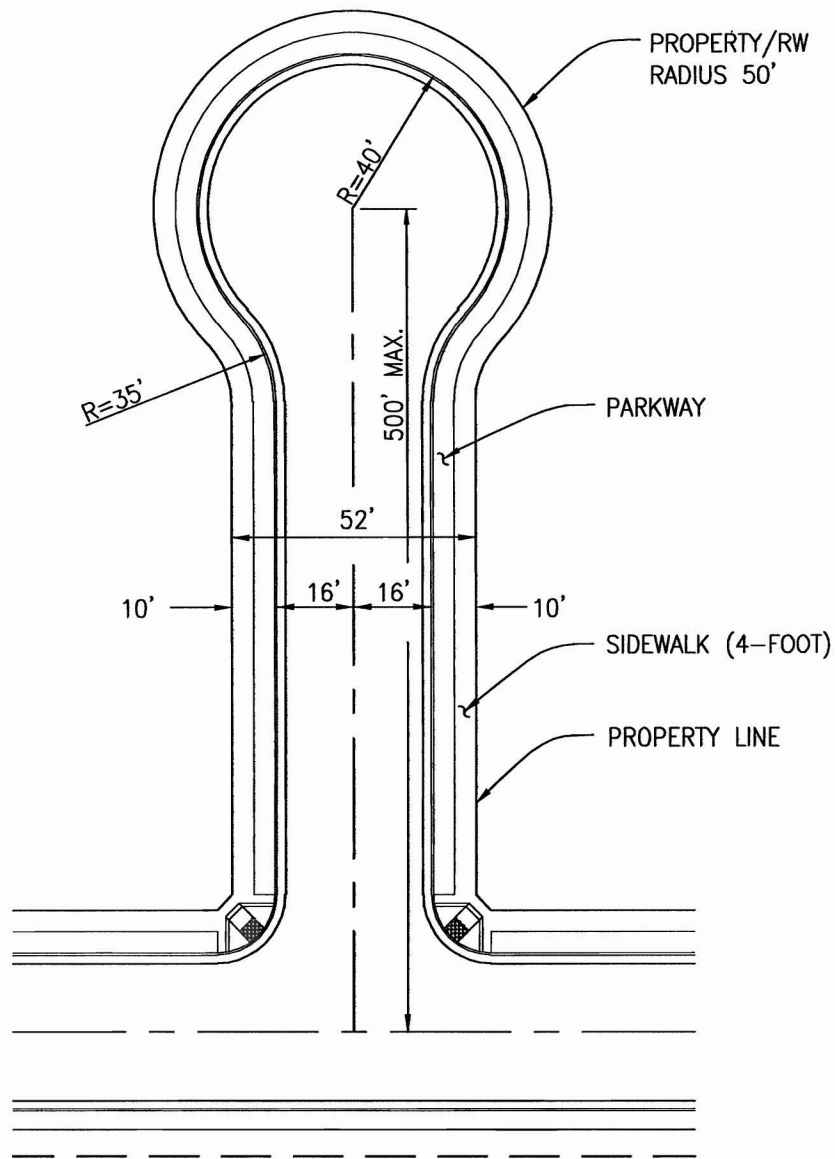
Ga. L. L. L.

DATE:

7/14/09

STD. DETAIL

3 - C



NOTES:

1. RADII SHOWN ARE FOR RESIDENTIAL CUL-DE-SACS. FOR COMMERCIAL OR INDUSTRIAL CUL-DE-SACS RADII SHALL BE APPROVED BY THE ENGINEER.
2. CUL-DE-SAC IS SHOWN WITH SEPARATED SIDEWALK. RIGHT-OF-WAY DIMENSIONS ARE THE SAME IF ATTACHED SIDEWALK IS USED.

CITY OF NEWMAN IMPROVEMENT STANDARDS



NON WALK THROUGH CUL-DE-SAC

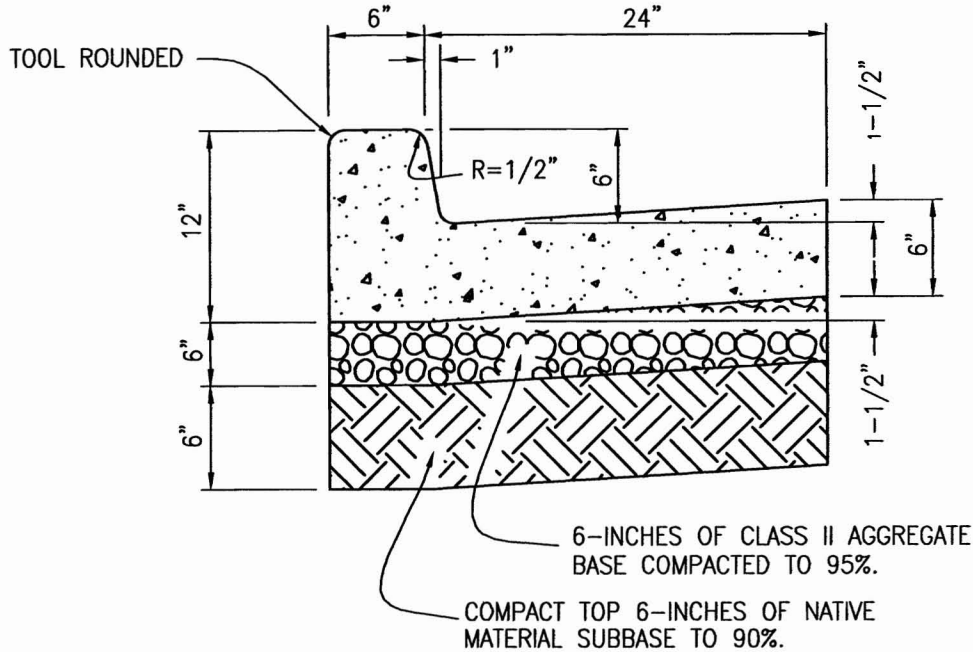
APPROVED BY:

G. R. G. G.

DATE: 7/14/09

STD. DETAIL

3 - D



STANDARD CURB AND GUTTER SECTION

NOTES:

1. CITY SHALL APPROVE SUBGRADE AND AGGREGATE BASE PREPARATION PRIOR TO PLACEMENT OF CONCRETE.
2. SOIL DENSITIES ARE EXPRESSED AS A PERCENTAGE OF MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D 1557.
3. IMMEDIATELY PRIOR TO PLACEMENT OF CONCRETE, THE GEOTECHNICAL ENGINEER SHALL CERTIFY THE MOISTURE CONTENT OF THE NATIVE MATERIAL SUBBASE HAS A MOISTURE CONTENT OF NOT LESS THAN 2% ABOVE OPTIMUM TO A DEPTH OF 18 INCHES. IN ADDITION, AGGREGATE BASE SHALL HAVE A MOISTURE CONTENT AS SPECIFIED BY THE DIRECTOR.

CITY OF NEWMAN IMPROVEMENT STANDARDS



STANDARD CURB & GUTTER SECTION

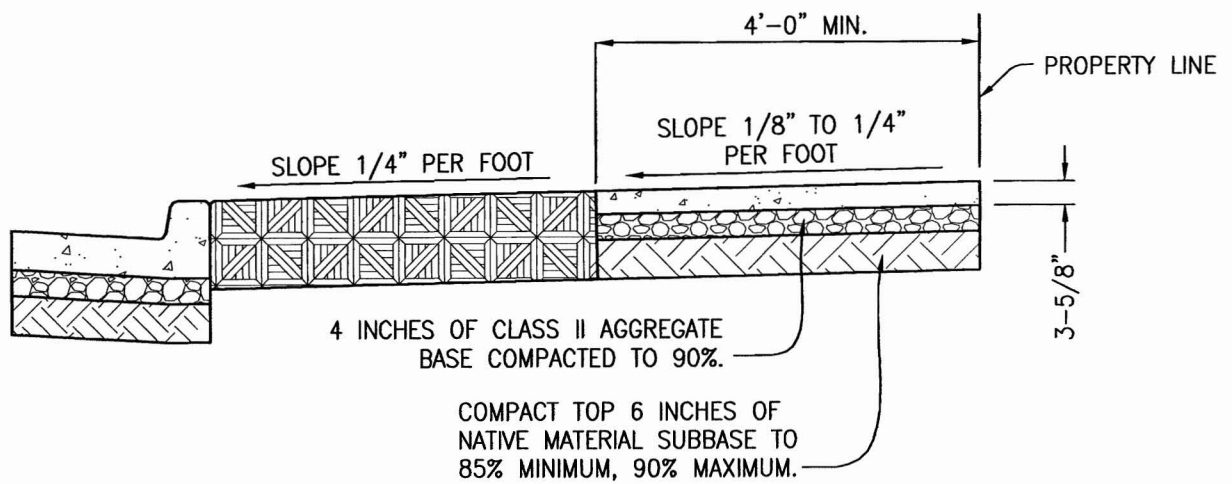
APPROVED BY:

San Lebed

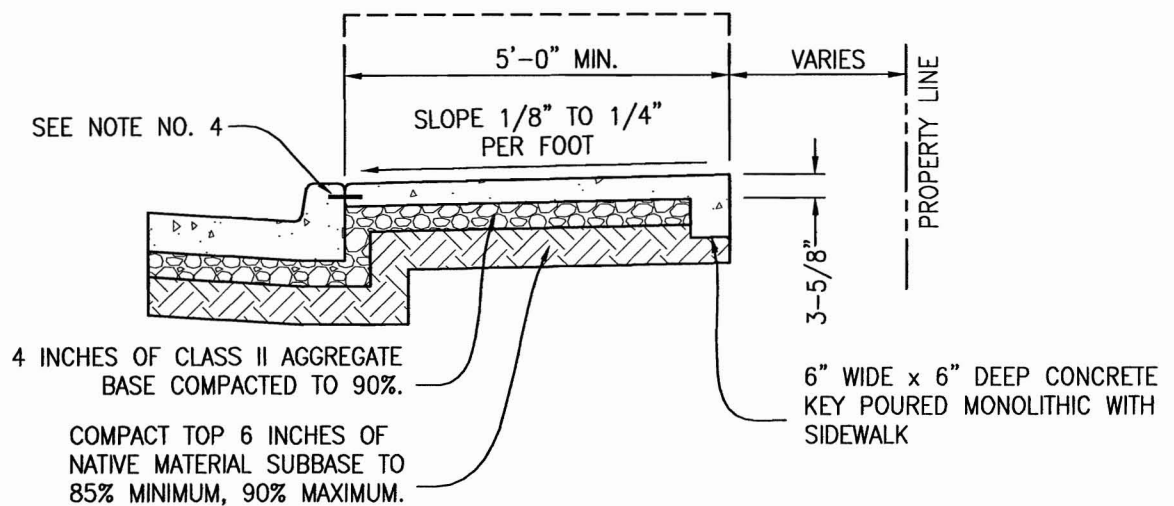
DATE: 7/14/09

STD. DETAIL

3 - E



SIDEWALK WITH PARKWAY



INTEGRAL SIDEWALK, CURB AND GUTTER

NOTES:

1. CITY SHALL APPROVE SUBGRADE AND AGGREGATE BASE PREPARATION PRIOR TO PLACEMENT OF CONCRETE.
2. SOIL DENSITIES ARE EXPRESSED AS A PERCENTAGE OF MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D 1557.
3. IMMEDIATELY PRIOR TO PLACEMENT OF CONCRETE, THE GEOTECHNICAL ENGINEER SHALL CERTIFY THE MOISTURE CONTENT OF THE NATIVE MATERIAL SUBBASE HAS A MOISTURE CONTENT OF NOT LESS THAN 3% ABOVE OPTIMUM TO A DEPTH OF 18 INCHES. IN ADDITION, AGGREGATE BASE SHALL HAVE A MOISTURE CONTENT AS SPECIFIED BY THE DIRECTOR.
4. CURB, GUTTER AND SIDEWALK SHALL NOT BE CONSTRUCTED MONOLITHICALLY. TIE THE SIDEWALK TO THE CURB WITH NO. 4 BARS @ 4' O.C.

CITY OF NEWMAN IMPROVEMENT STANDARDS

SIDEWALK, CURB & GUTTER SECTION



APPROVED BY:

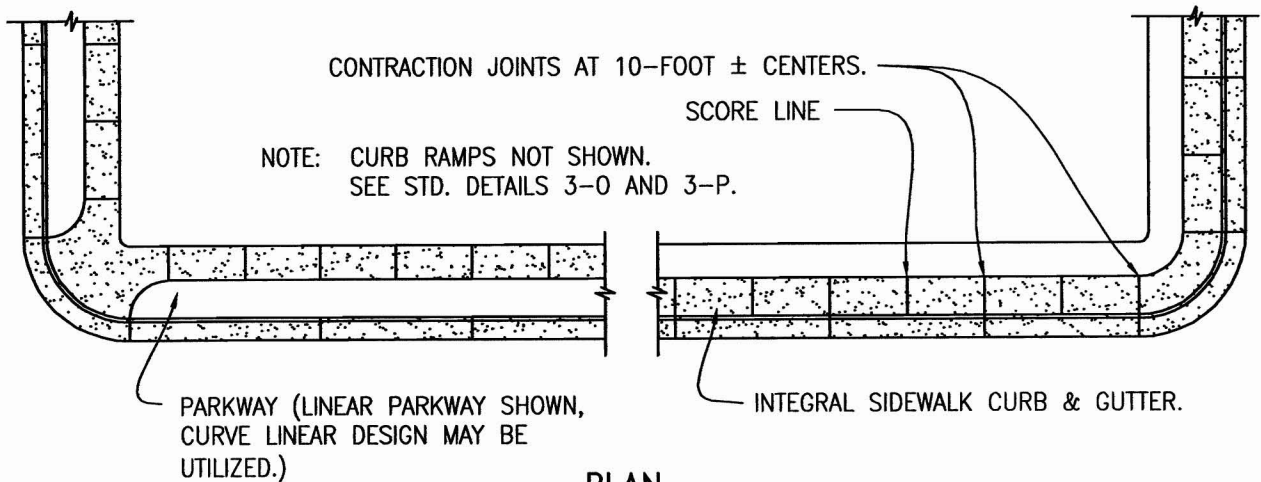
G. R. R. R.

DATE:

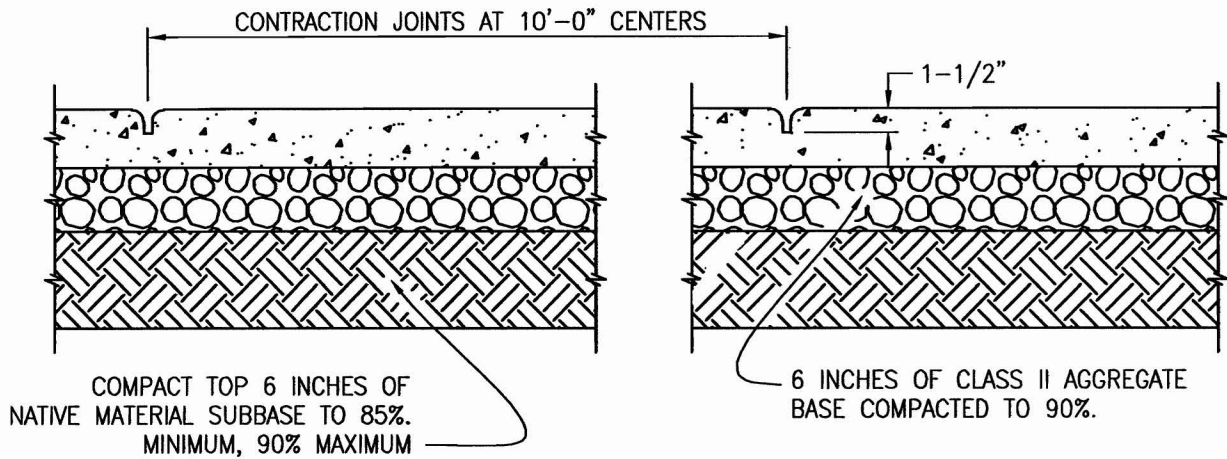
7/14/09

STD. DETAIL

3 - F



PLAN
TYPICAL CITY BLOCK



LONGITUDINAL SECTIONS

NOTES:

1. EDGES TO HAVE 1/2-INCH RADIUS.
2. WHEN BREAKING OUT SIDEWALK, CONCRETE SHALL BE REMOVED TO THE NEAREST CONTRACTION JOINT/SCORE LINE.
3. SOIL DENSITIES ARE EXPRESSED AS A PERCENTAGE OF MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D 1557.
4. CITY SHALL APPROVE SUBGRADE AND AGGREGATE BASE PREPARATION PRIOR TO PLACEMENT OF CONCRETE.
5. SEE STD. DETAIL 3-F FOR SUBGRADE AND BASE MOISTURE REQUIREMENTS.

CITY OF NEWMAN IMPROVEMENT STANDARDS

SIDEWALK CONSTRUCTION



APPROVED BY:

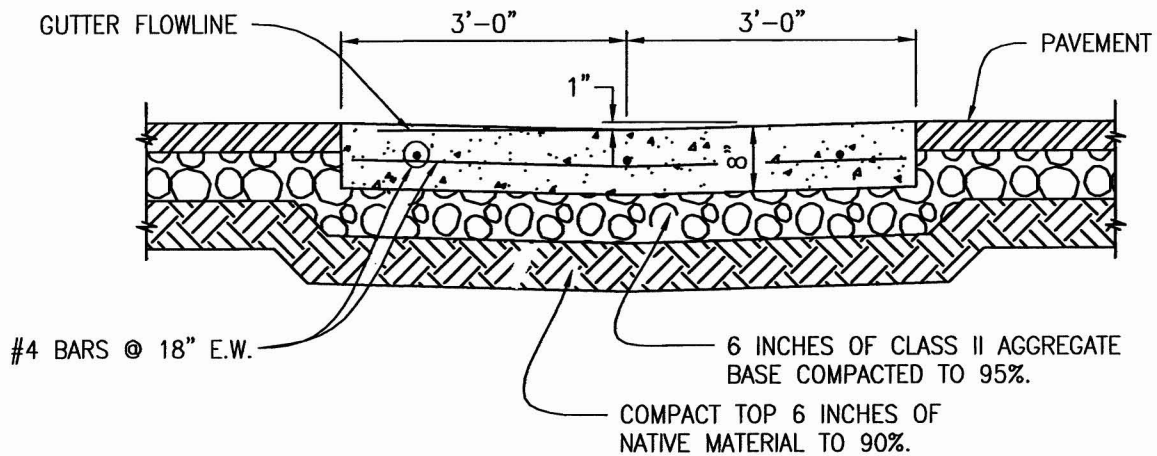
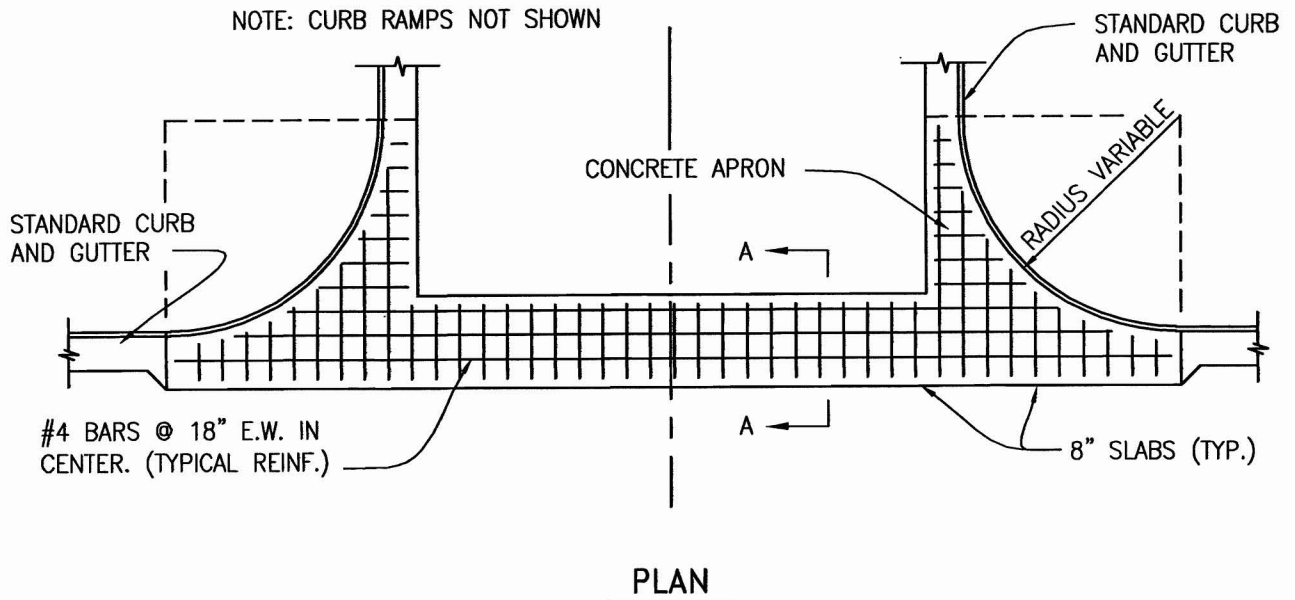
Ga R. Gue

DATE:

7/14/09

STD. DETAIL

3 - G



NOTES:

1. MINIMUM DIFFERENCE IN ELEVATION ACROSS GUTTER TO BE 0.30 FEET.
2. BOTH APRONS AND GUTTER SHALL BE PLACED OVER 6 INCHES OF CLASS II AGGREGATE BASE COMPACTED TO 95%.
3. SOIL DENSITIES ARE EXPRESSED AS A PERCENTAGE OF MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D 1557.
4. SEE STD. DETAIL 3-F FOR SUBGRADE AND BASE MOISTURE REQUIREMENTS.

CITY OF NEWMAN IMPROVEMENT STANDARDS



VALLEY CROSS GUTTER

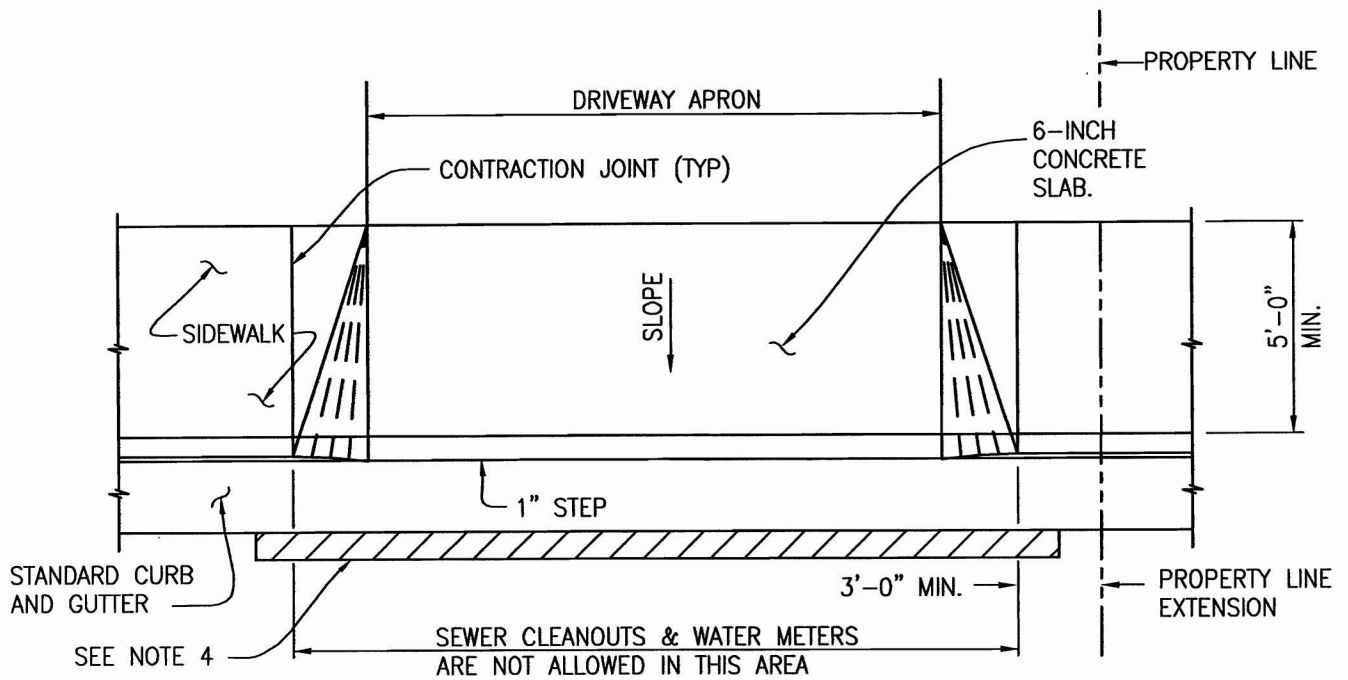
APPROVED BY:

G. Lopez

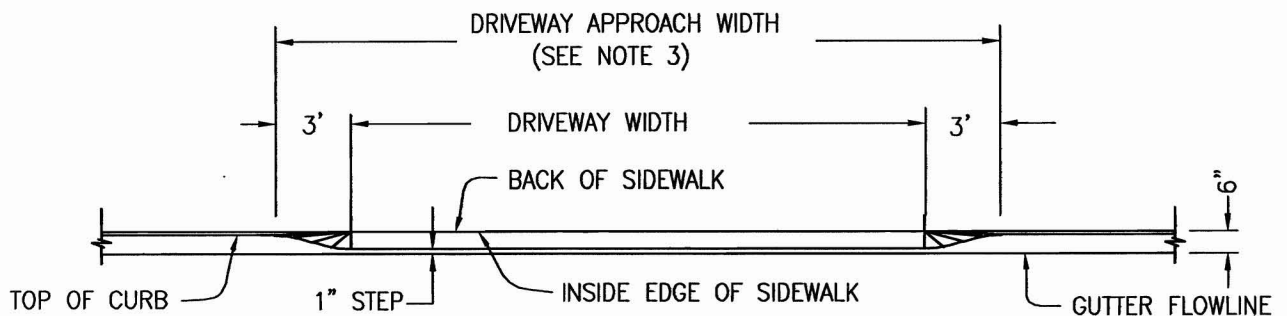
DATE: 7/14/09

STD. DETAIL

3 - H



PLAN



ELEVATION

NOTES:

1. DRIVEWAY APPROACHES SHALL BE INSTALLED OVER 6 INCHES OF CLASS II AGGREGATE BASE COMPACTED TO 90% OVER 6 INCHES OF NATIVE MATERIAL SUBBASE COMPACTED TO 85% MINIMUM, 90% MAXIMUM.
2. SOIL DENSITIES ARE EXPRESSED AS A PERCENTAGE OF MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D 1557.
3. DRIVEWAY LOCATION AND MAXIMUM DRIVEWAY APPROACH WIDTH SHALL CONFORM TO SECTIONS 3.13 AND 3.14 OF THESE STANDARDS.
4. WHEN INSTALLING A NEW DRIVEWAY IN EXISTING CURB, GUTTER AND SIDEWALK, SAWCUT TOP AND FACE OF CURB, GUTTER AND SIDEWALK TO A MINIMUM DEPTH OF 2 INCHES PRIOR TO REMOVAL. CHIP CONCRETE TO A VERTICAL PLANE BELOW SAWCUT. ALSO, SAWCUT, REMOVE AND REPLACE EXISTING A.C. IN FRONT OF DRIVEWAY AS REQUIRED BY PUBLIC WORKS DIRECTOR.
5. SEE STD. DETAIL 3-F FOR SUBGRADE AND BASE MOISTURE REQUIREMENTS.

CITY OF NEWMAN IMPROVEMENT STANDARDS

RESIDENTIAL DRIVEWAY-ATTACHED SIDEWALK



APPROVED BY:

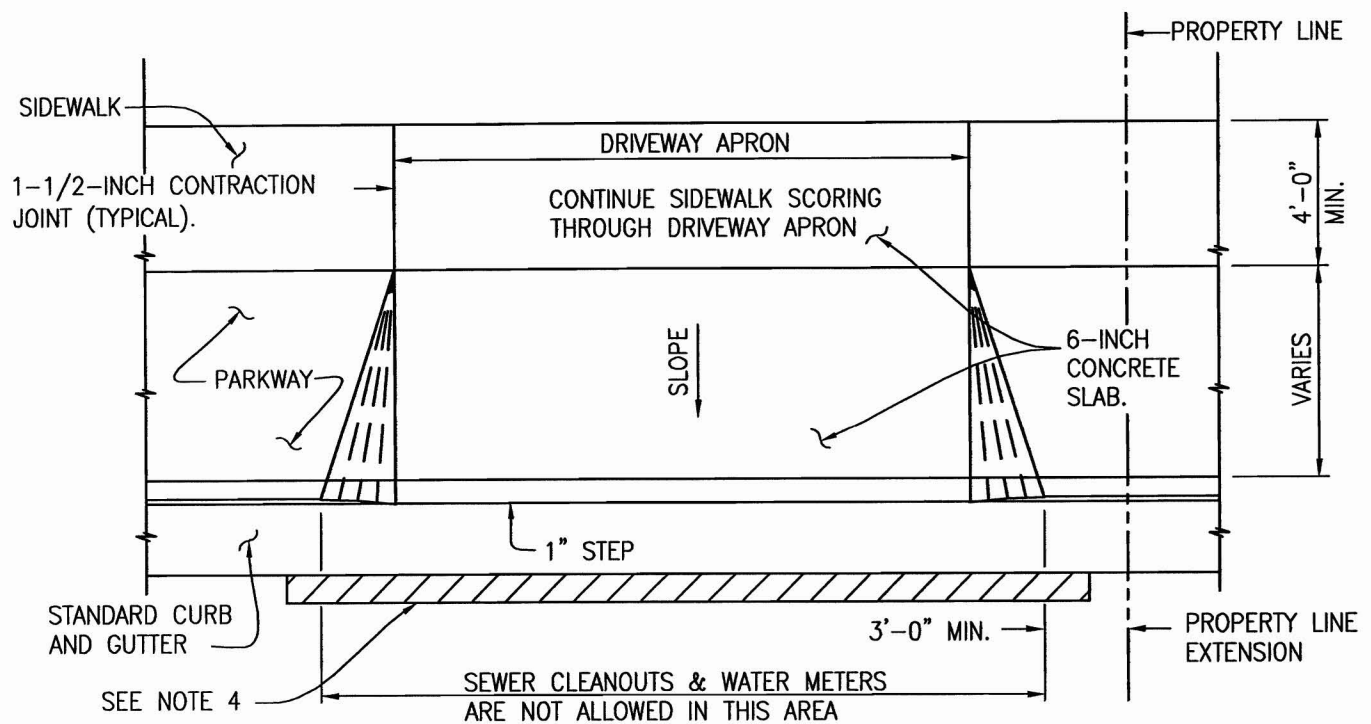
Gra R. [Signature]

DATE:

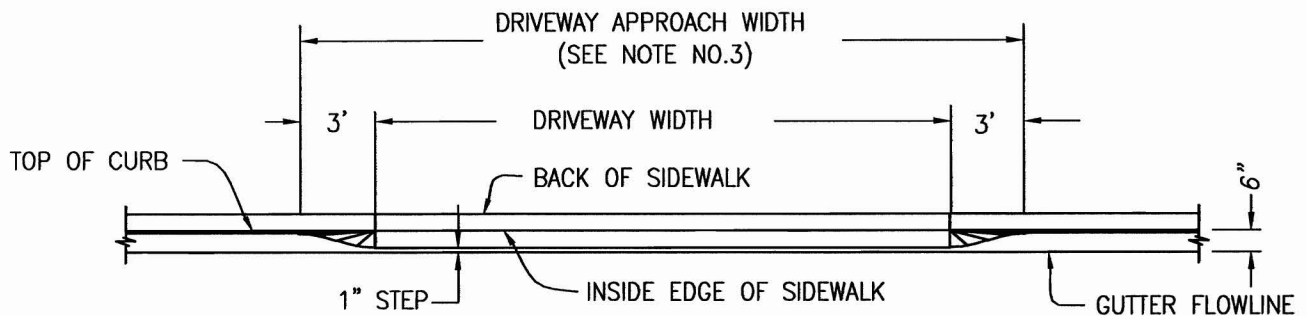
7/14/09

STD. DETAIL

3 - 1



PLAN



ELEVATION

NOTES:

1. DRIVEWAY APPROACHES SHALL BE INSTALLED OVER 6 INCHES OF CLASS II AGGREGATE BASE COMPACTED TO 90% OVER 6 INCHES OF NATIVE MATERIAL SUBBASE COMPACTED TO 85% MINIMUM, 90% MAXIMUM.
2. SOIL DENSITIES ARE EXPRESSED AS A PERCENTAGE OF MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D 1557.
3. DRIVEWAY LOCATION AND MAXIMUM DRIVEWAY APPROACH WIDTH SHALL CONFORM TO SECTIONS 3.13 AND 3.14 OF THESE STANDARDS.
4. WHEN INSTALLING A NEW DRIVEWAY IN EXISTING CURB, GUTTER AND SIDEWALK, SAWCUT TOP AND FACE OF CURB, GUTTER AND SIDEWALK TO A MINIMUM DEPTH OF 2 INCHES PRIOR TO REMOVAL. CHIP CONCRETE TO A VERTICAL PLANE BELOW SAWCUT. ALSO, SAWCUT, REMOVE AND REPLACE EXISTING A.C. IN FRONT OF DRIVEWAY AS REQUIRED BY PUBLIC WORKS DIRECTOR.
5. SEE STD. DETAIL 3-F FOR SUBGRADE AND BASE MOISTURE REQUIREMENTS.

CITY OF NEWMAN IMPROVEMENT STANDARDS



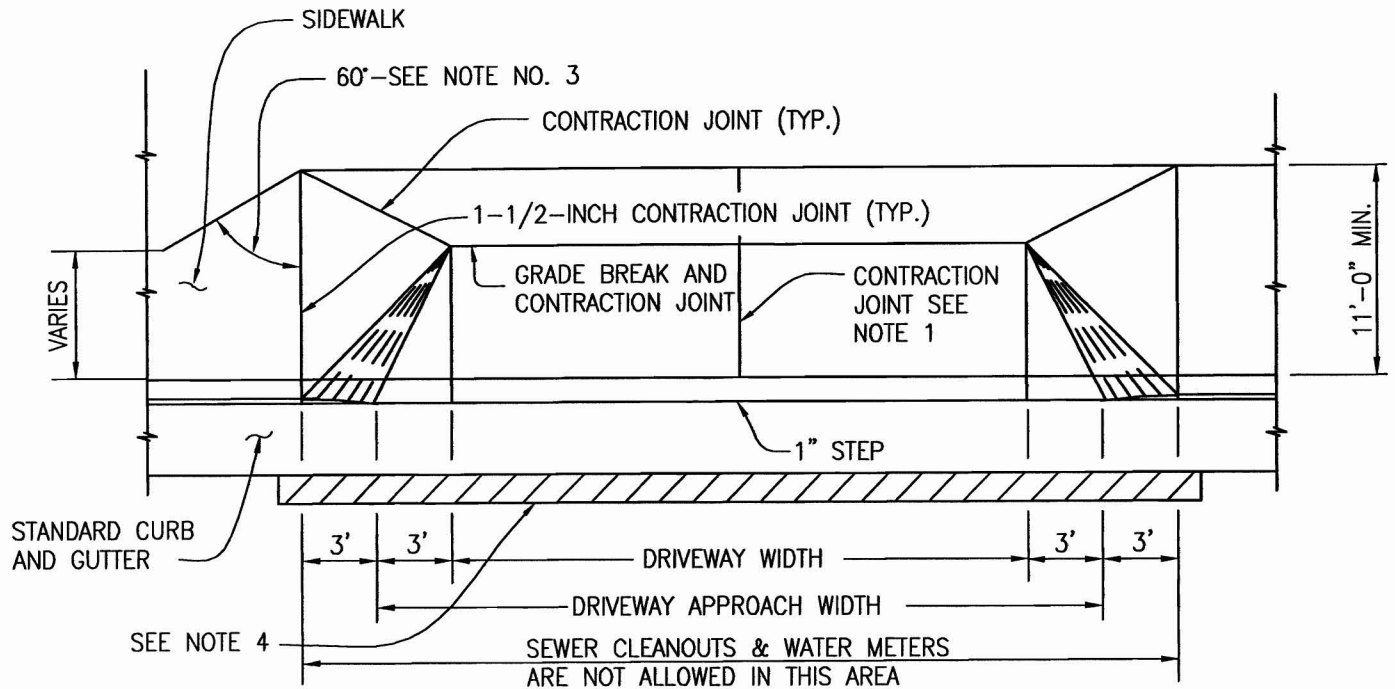
RESIDENTIAL DRIVEWAY-SEPARATED SIDEWALK

APPROVED BY:

Grady DATE: 7/14/09

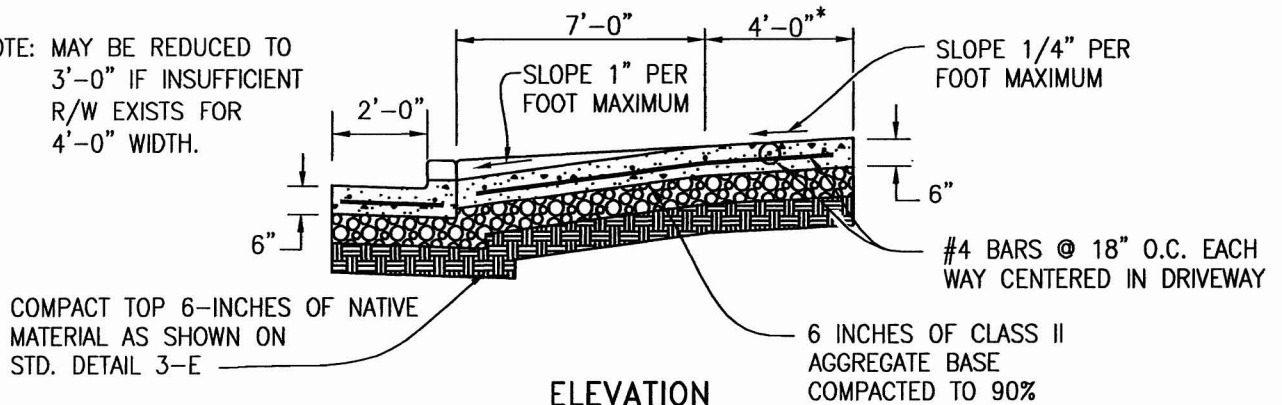
STD. DETAIL

3 - J



PLAN

* NOTE: MAY BE REDUCED TO 3'-0" IF INSUFFICIENT R/W EXISTS FOR 4'-0" WIDTH.



ELEVATION

NOTES:

1. CONTRACTION JOINT TO BE ADDED TO CENTER OF APPROACH IF IT IS OVER 20 FEET WIDE.
2. SOIL DENSITIES ARE EXPRESSED AS A PERCENTAGE OF MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D 1557.
3. PROVIDE TRANSITION TO SIDEWALKS THAT ARE LESS THAN 11 FEET IN WIDTH.
4. WHEN INSTALLING A NEW DRIVEWAY IN EXISTING CURB, GUTTER, AND SIDEWALK, SAWCUT TOP AND FACE OF CURB, GUTTER AND SIDEWALK TO A MINIMUM DEPTH OF 2 INCHES PRIOR TO REMOVAL. CHIP CONCRETE TO A VERTICAL PLANE BELOW SAWCUT. ALSO SAWCUT, REMOVE AND REPLACE EXISTING A.C. IN FRONT OF DRIVEWAY AS REQUIRED BY PUBLIC WORKS DIRECTOR.
5. SEE STD. DETAIL 3-F FOR SUBGRADE AND BASE MOISTURE REQUIREMENTS.

CITY OF NEWMAN IMPROVEMENT STANDARDS



COMMERCIAL/INDUSTRIAL DRIVEWAY

APPROVED BY: *[Signature]*

DATE: 7/14/09

STD. DETAIL

3 - K

NOTE: R-10' MINIMUM
R-30' MAXIMUM

OPTIONAL ON-SITE SIDEWALK.

10' TRANSITION
FROM SIDEWALK.

NOMINAL DRIVEWAY WIDTH

AC OR CONCRETE
DRIVEWAY

NOTE: ALTERNATIVE DESIGN
AND PLACEMENT OF
WHEEL CHAIR RAMP
ARE ALLOWED
(SEE NOTE 3.)

6" CONCRETE CURB

SIDEWALK

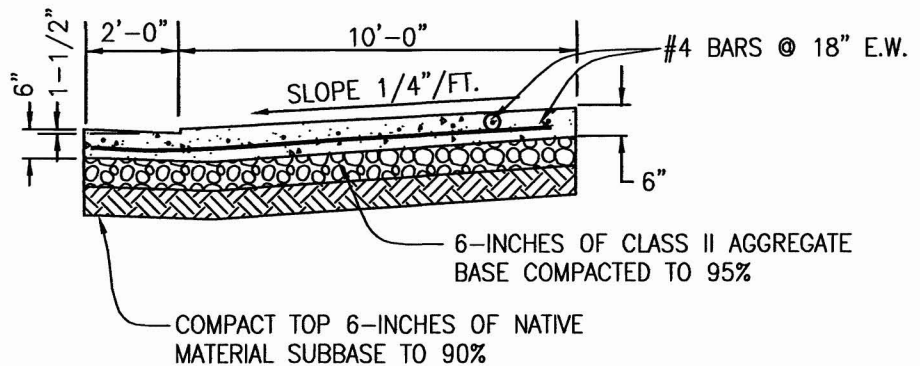
COLD JOINT

1" STEP

STANDARD CURB
AND GUTTER

CONTRACTION JOINT

PLAN



SECTION A-A

NOTES:

1. SOIL DENSITIES ARE EXPRESSED AS A PERCENTAGE OF MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D 1557.
2. PROVIDE TYPE 'A' CATCH BASIN ON UPSTREAM SIDE OF DRIVEWAY APPROACH IF DEPTH OF FLOW IN GUTTER EXCEEDS 3 INCHES.
3. CURB RAMP TYPE USED SHALL BE DETERMINED BASED ON DIMENSIONS OF CROSSWALK AND TRAVEL PATHS. ALL RAMPS SHALL MEET THE STATE ACCESSIBILITY STANDARDS.
4. SEE STD. DETAIL 3-F FOR SUBGRADE AND BASE MOISTURE REQUIREMENTS.

CITY OF NEWMAN IMPROVEMENT STANDARDS

SPECIAL COMMERCIAL DRIVEWAY

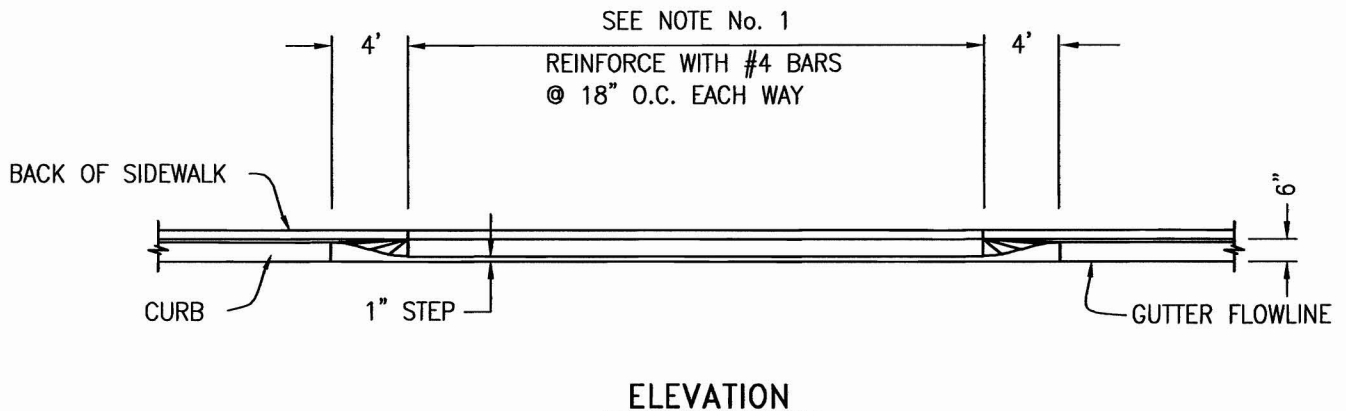
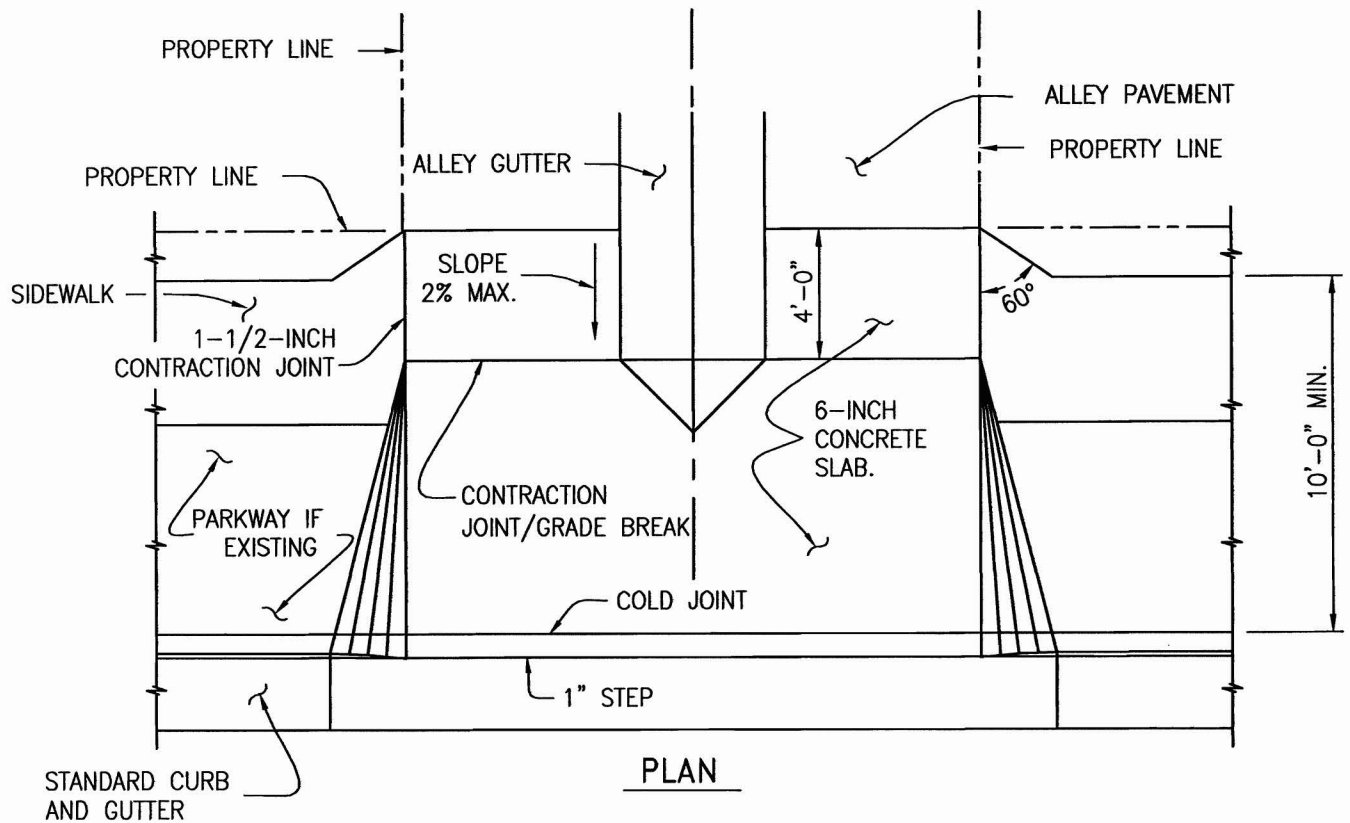


APPROVED BY: *G. R. R. R.*

DATE: 7/14/09

STD. DETAIL

3 - L



NOTES:

1. ALLEY APPROACHES SHALL BE INSTALLED OVER 6 INCHES OF CLASS II AGGREGATE BASE COMPACTED TO 95%, OVER 6 INCHES OF NATIVE MATERIAL SUBBASE COMPACTED TO 90%.
2. SOIL DENSITIES ARE EXPRESSED AS A PERCENTAGE OF MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D 1557.
3. ALLEY SHALL BE SYMMETRICAL ABOUT CENTERLINE.
4. SEE STD. DETAIL 3-F FOR SUBGRADE AND BASE MOISTURE REQUIREMENTS.

CITY OF NEWMAN IMPROVEMENT STANDARDS



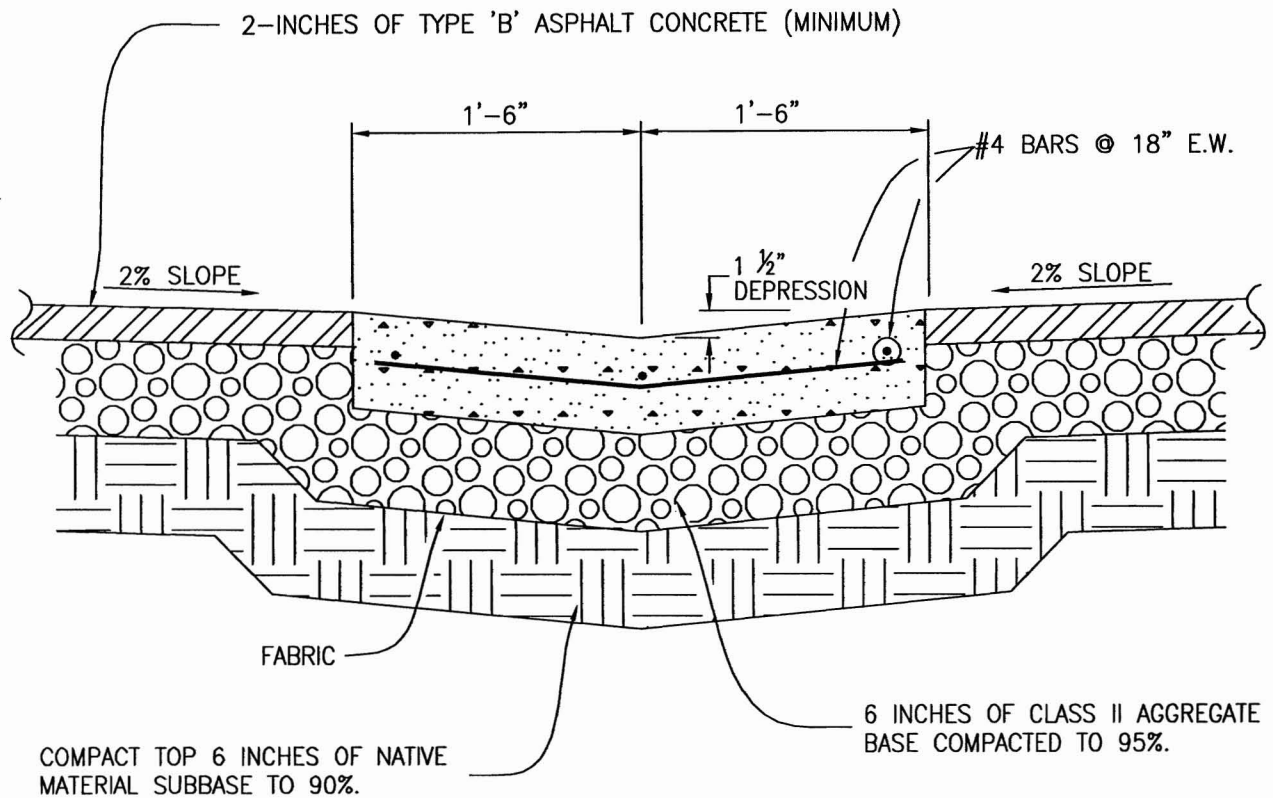
ALLEY APPROACH DETAILS

APPROVED BY: *Ga R. G. G.*

DATE: 7/14/09

STD. DETAIL

3 - M



SECTION

NOTES:

1. ALLEY GUTTER SHALL BE 6 INCHES THICK.
2. CITY SHALL APPROVE SUBGRADE AND AGGREGATE BASE PRIOR TO PLACEMENT OF CONCRETE.
3. SOIL DENSITIES ARE EXPRESSED AS A PERCENTAGE OF MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D 1557.

CITY OF NEWMAN IMPROVEMENT STANDARDS

ALLEY GUTTER AND PAVEMENT SECTION



APPROVED BY: *G. K. B. G.*

DATE: 7/14/09

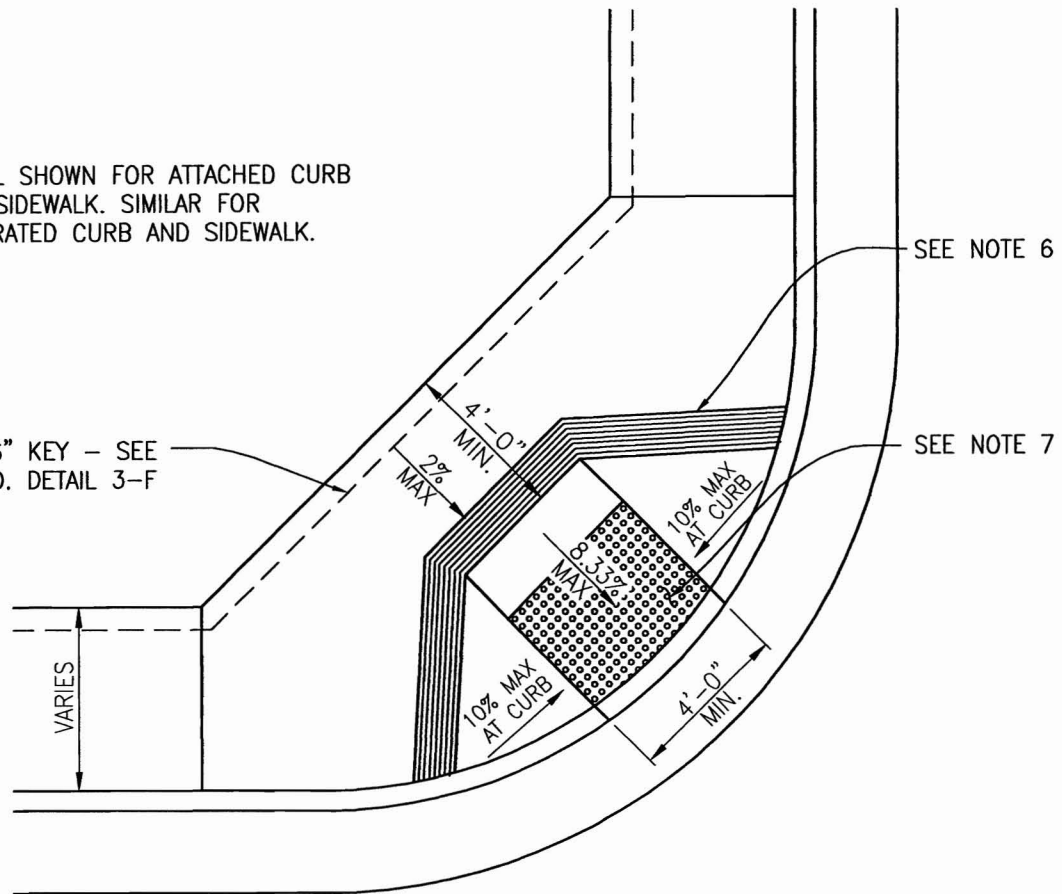
STD. DETAIL

3 - N

NOTE:

DETAIL SHOWN FOR ATTACHED CURB
AND SIDEWALK. SIMILAR FOR
SEPARATED CURB AND SIDEWALK.

6" x 6" KEY - SEE
STD. DETAIL 3-F



NOTES:

1. CURB RAMP TYPE 1 IS FOR NEW CONSTRUCTION.
2. SIDEWALK AND CURB RAMP IN RETURN AREA SHALL BE 5 1/2 INCHES THICK MINIMUM.
3. TOP OF CURB SHALL BE FLUSH WITH GUTTER FLOWLINE AT RAMP.
4. PLACE 6 INCHES OF CLASS II AGGREGATE BASE COMPACTED TO 95% OVER 6 INCHES OF NATIVE MATERIAL SUBBASE COMPACTED TO 90% UNDER RAMP. SEE STD. DETAIL 3-E FOR SUBGRADE AND BASE MOISTURE REQUIREMENTS.
5. SOIL DENSITIES ARE EXPRESSED AS A PERCENTAGE OF MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D1557.
6. THE CURB RAMP SHALL BE OUTLINED, AS SHOWN, WITH A 1-FOOT WIDE BORDER WITH 1/4-INCH WIDE GROOVES 3/4-INCH ON CENTER. GROOVES SHALL BE 1/4 INCH DEEP.
7. DETECTABLE WARNING SURFACE (TRUNCATED DOMES) SHALL EXTEND THE FULL WIDTH AND 3 FEET DEEP ON THE RAMP. THE DETECTABLE WARNING SURFACE SHALL MEET THE LATEST REQUIREMENTS OF THE AMERICANS WITH DISABILITIES ACT (ADA). THE EDGE OF THE DETECTABLE WARNING SURFACE NEAREST THE STREET SHALL BE BETWEEN 6 INCHES AND 9 INCHES OF THE GUTTER FLOWLINE.
8. THE SLOPE AT BOTTOM OF RAMP, INCLUDING GUTTER, SHALL NOT EXCEED 5% FOR 4 FEET FROM GUTTER FLOWLINE.

CITY OF NEWMAN IMPROVEMENT STANDARDS

CURB RAMP - TYPE 1



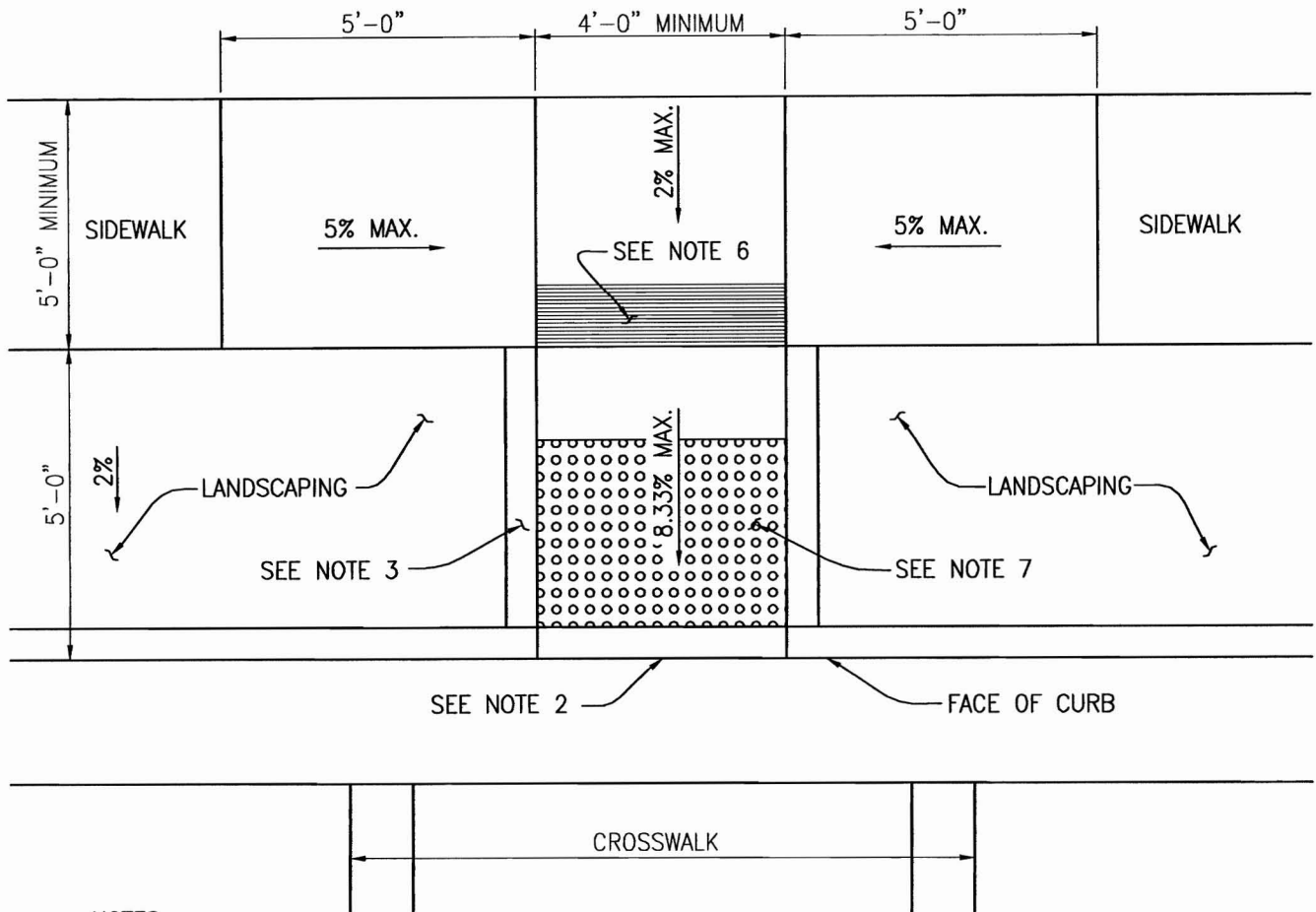
APPROVED BY:

G. L. Boyd

DATE: 7/14/09

STD. DETAIL

3 - 0



NOTES:

1. CURB RAMP TYPE 3 IS FOR NEW CONSTRUCTION.
2. TOP OF CURB SHALL BE FLUSH WITH GUTTER FLOWLINE AT RAMP.
3. 6-INCH WIDE BY 12-INCH DEEP CURB. TOP OF CURB SHALL BE FLUSH WITH RAMP LANDING AT SIDEWALK AND 6 INCHES ABOVE GUTTER FLOW LINE AT CURB AND GUTTER.
4. PLACE 6 INCHES OF CLASS II AGGREGATE BASE COMPACTED TO 90% OVER 6 INCHES OF NATIVE MATERIAL SUBBASE COMPACTED TO 85% MINIMUM, 90% MAXIMUM UNDER RAMP. SEE STD. DETAIL 3-F FOR SUBGRADE AND BASE MOISTURE REQUIREMENTS.
5. SOIL DENSITIES ARE EXPRESSED AS A PERCENTAGE OF MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM 1557.
6. THE CURB RAMP SHALL BE OUTLINED, AS SHOWN, WITH A 1-FOOT WIDE BORDER WITH $\frac{1}{4}$ -INCH WIDE GROOVES $\frac{3}{4}$ -INCH ON CENTER. GROOVES SHALL BE $\frac{1}{4}$ INCH DEEP.
7. DETECTABLE WARNING SURFACE (TRUNCATED DOMES) SHALL EXTEND THE FULL WIDTH AND 3 FEET DEEP ON THE RAMP. THE DETECTABLE WARNING SURFACE SHALL MEET THE LATEST REQUIREMENTS OF THE AMERICANS WITH DISABILITIES ACT (ADA). THE EDGE OF THE DETECTABLE WARNING SURFACE NEAREST THE STREET SHALL BE BETWEEN 6 INCHES AND 9 INCHES OF THE GUTTER FLOWLINE.
8. THE SLOPE AT BOTTOM OF RAMP, INCLUDING GUTTER, SHALL NOT EXCEED 5% FOR 4 FEET FROM GUTTER FLOWLINE.

CITY OF NEWMAN IMPROVEMENT STANDARDS

CURB RAMP - TYPE 3



APPROVED BY: *G. R. R. R.*

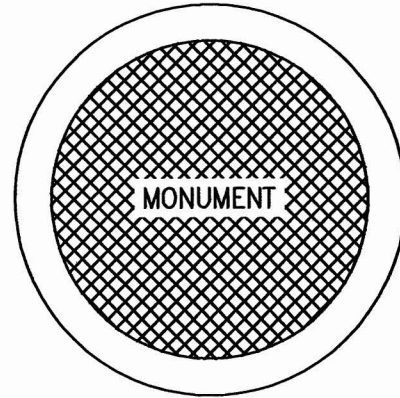
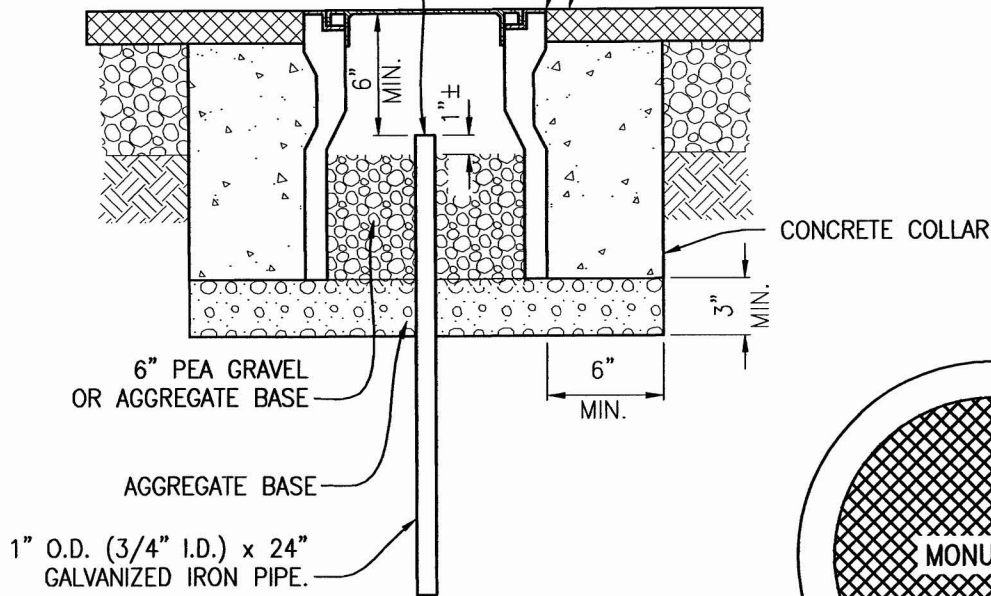
DATE: 7/14/09

STD. DETAIL

3 - P

MARKED OR TAGGED WITH
CERTIFICATE NUMBER OF
L.S. OR APPROVED R.C.E.

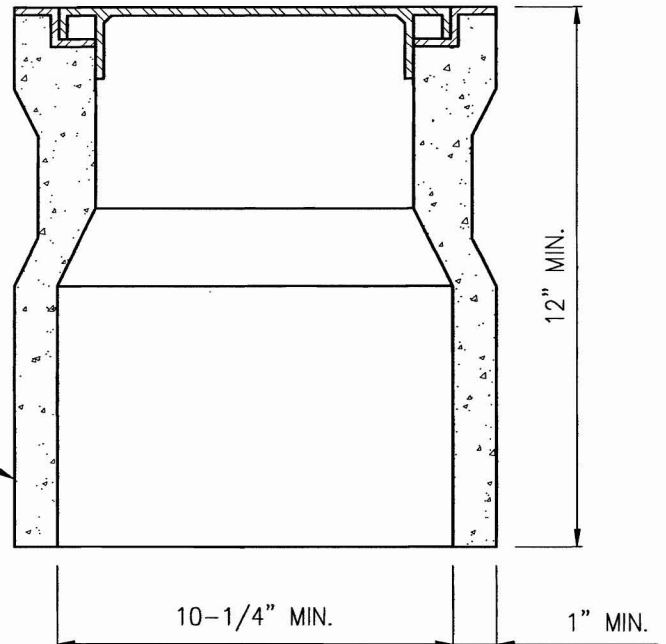
FINISHED GRADE
ASPHALT CONCRETE



NOTE:

1. FOR INSTALLATION INSIDE STREET SECTIONS ONLY.

CHRISTY G5 OR BROOKS 3RT
TRAFFIC BOX.



CITY OF NEWMAN IMPROVEMENT STANDARDS



SURVEY MONUMENT AND WELL

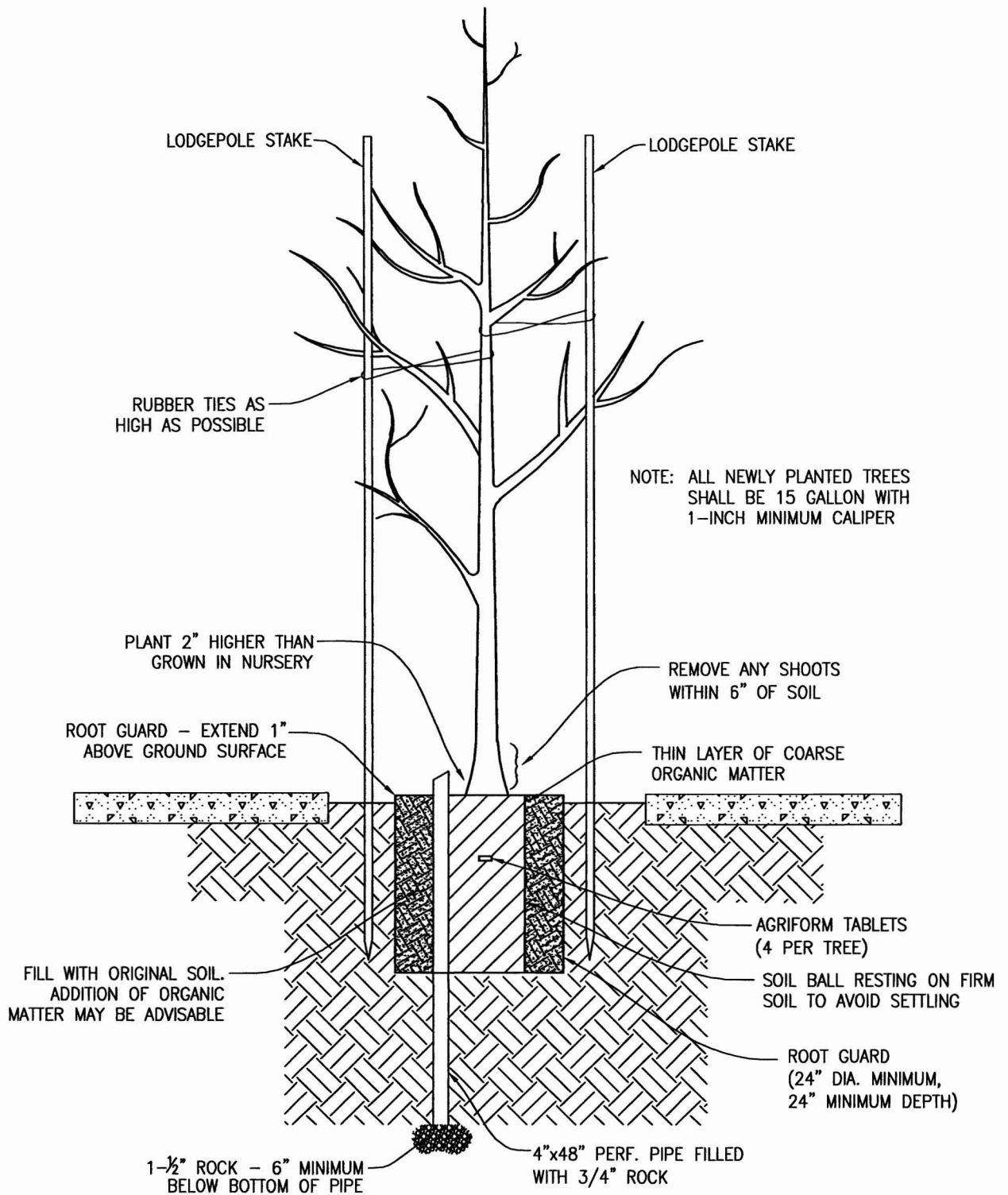
APPROVED BY:

Go. K. G. G.

DATE: 7/14/09

STD. DETAIL

3 - Q



CITY OF NEWMAN IMPROVEMENT STANDARDS

TREE PLANTING



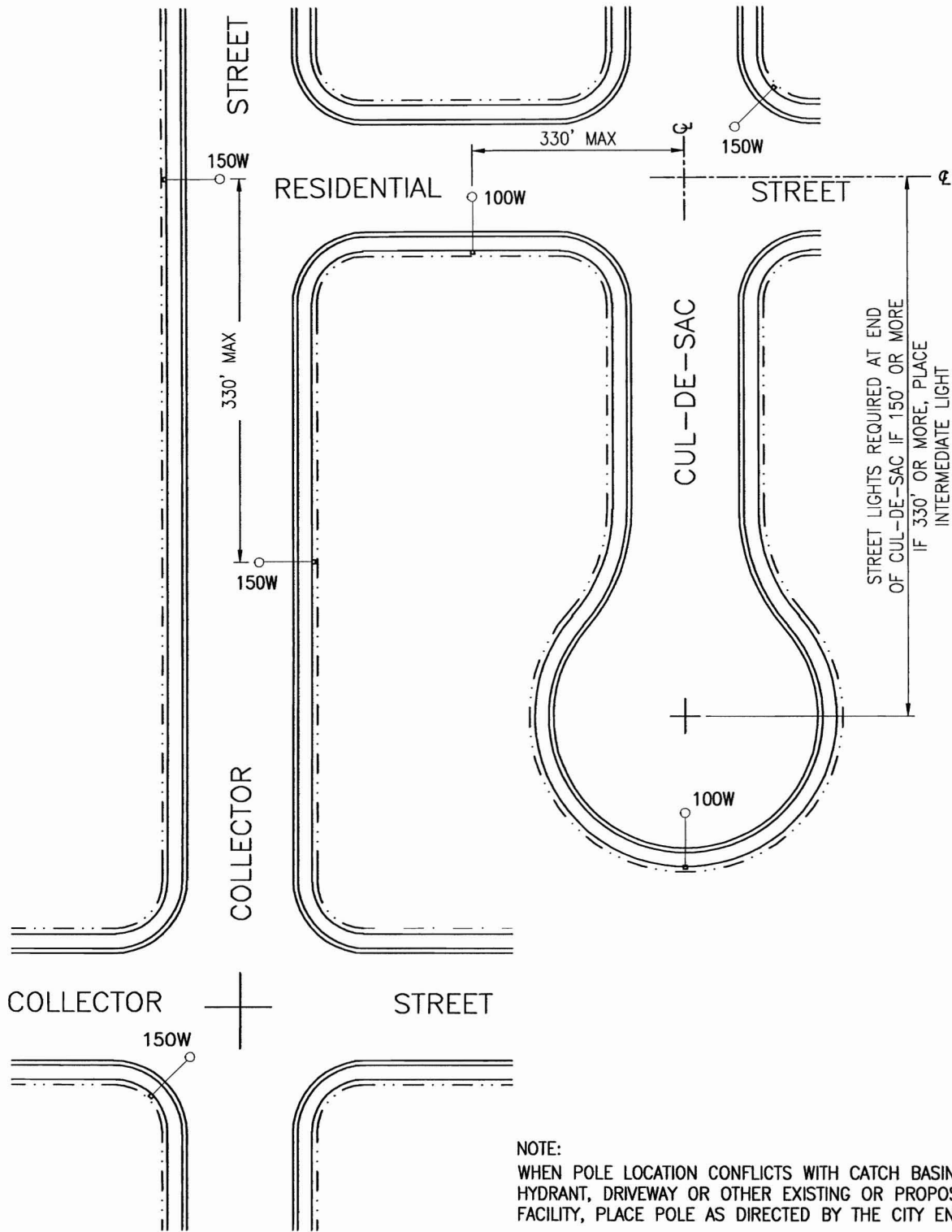
APPROVED BY:

Ga. R. R. R.

DATE: 7/14/09

STD. DETAIL

3 - R



NOTE:
WHEN POLE LOCATION CONFLICTS WITH CATCH BASIN, FIRE HYDRANT, DRIVEWAY OR OTHER EXISTING OR PROPOSED FACILITY, PLACE POLE AS DIRECTED BY THE CITY ENGINEER.

CITY OF NEWMAN IMPROVEMENT STANDARDS



ELECTROLIER LOCATIONS

APPROVED BY:

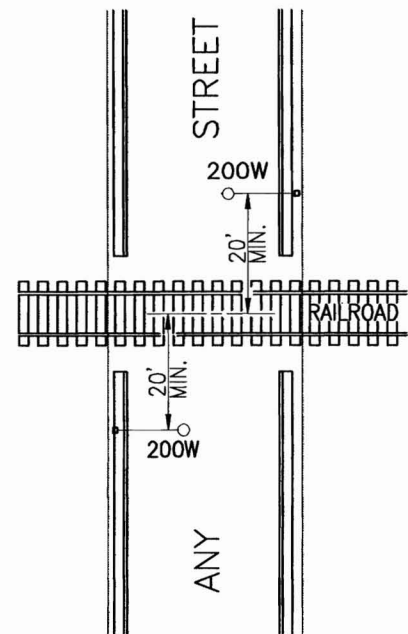
G. R. [Signature]

DATE:

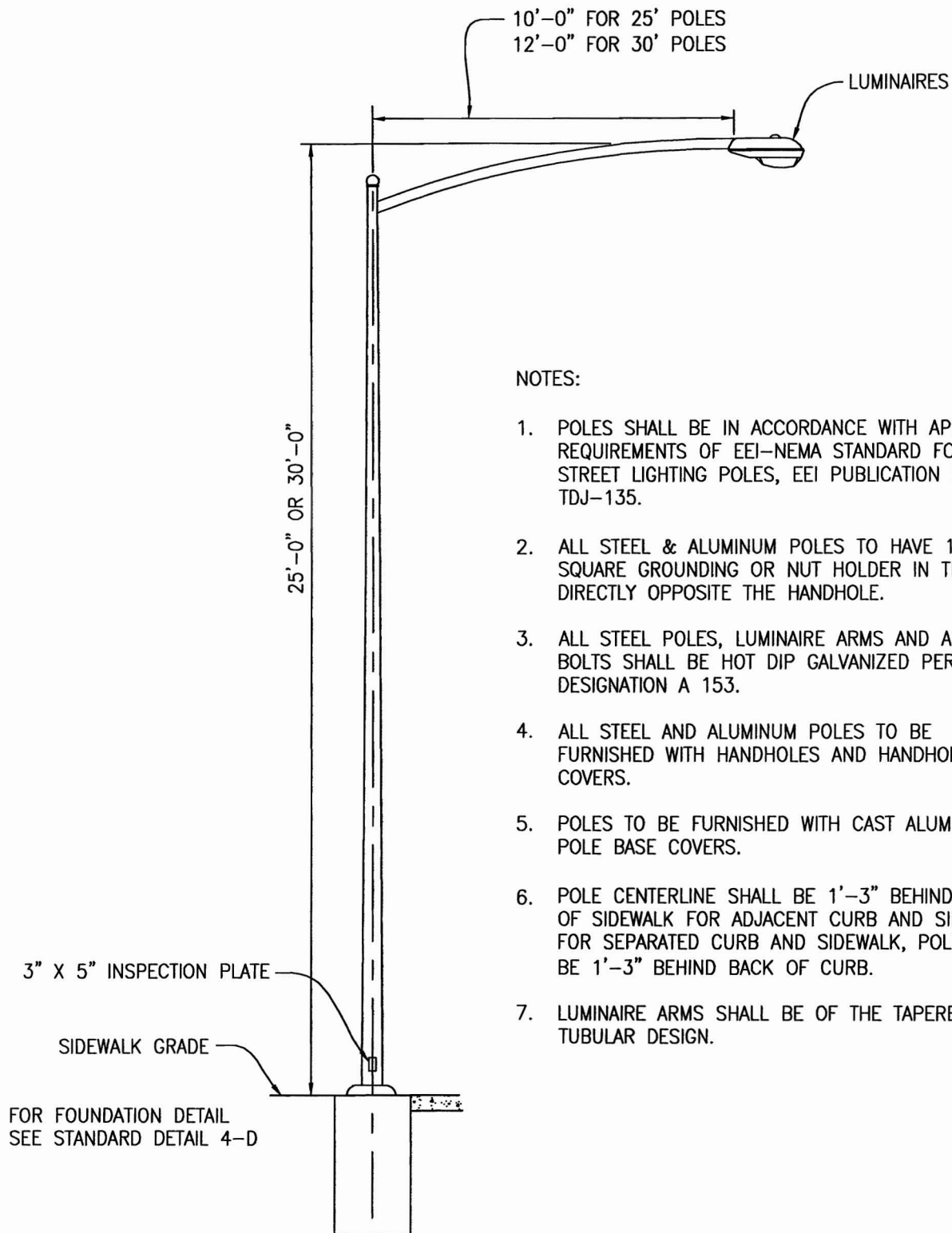
7/14/09

STD. DETAIL

4 - A



WHEN POLE LOCATION CONFLICTS WITH CATCH BASIN, FIRE
HYDRANT, DRIVEWAY OR OTHER EXISTING OR PROPOSED FACILITY,
PLACE POLE AS DIRECTED BY THE CITY ENGINEER.



NOTES:

1. POLES SHALL BE IN ACCORDANCE WITH APPLICABLE REQUIREMENTS OF EEI-NEMA STANDARD FOR STREET LIGHTING POLES, EEI PUBLICATION NO. TDJ-135.
2. ALL STEEL & ALUMINUM POLES TO HAVE 1/2" SQUARE GROUNDING OR NUT HOLDER IN THE POLE DIRECTLY OPPOSITE THE HANDHOLE.
3. ALL STEEL POLES, LUMINAIRE ARMS AND ANCHOR BOLTS SHALL BE HOT DIP GALVANIZED PER ASTM DESIGNATION A 153.
4. ALL STEEL AND ALUMINUM POLES TO BE FURNISHED WITH HANDHOLES AND HANDHOLE COVERS.
5. POLES TO BE FURNISHED WITH CAST ALUMINUM POLE BASE COVERS.
6. POLE CENTERLINE SHALL BE 1'-3" BEHIND BACK OF SIDEWALK FOR ADJACENT CURB AND SIDEWALK. FOR SEPARATED CURB AND SIDEWALK, POLE SHALL BE 1'-3" BEHIND BACK OF CURB.
7. LUMINAIRE ARMS SHALL BE OF THE TAPERED TUBULAR DESIGN.

CITY OF NEWMAN IMPROVEMENT STANDARDS



ELECTROLIER INSTALLATION

APPROVED BY:

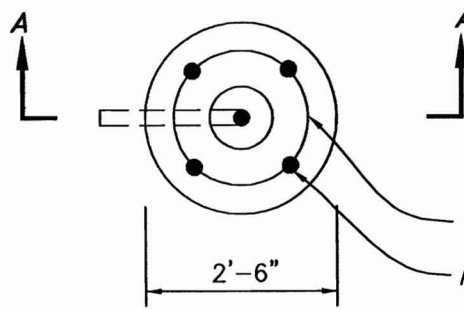
G. d. h. d.

DATE:

7/14/09

STD. DETAIL

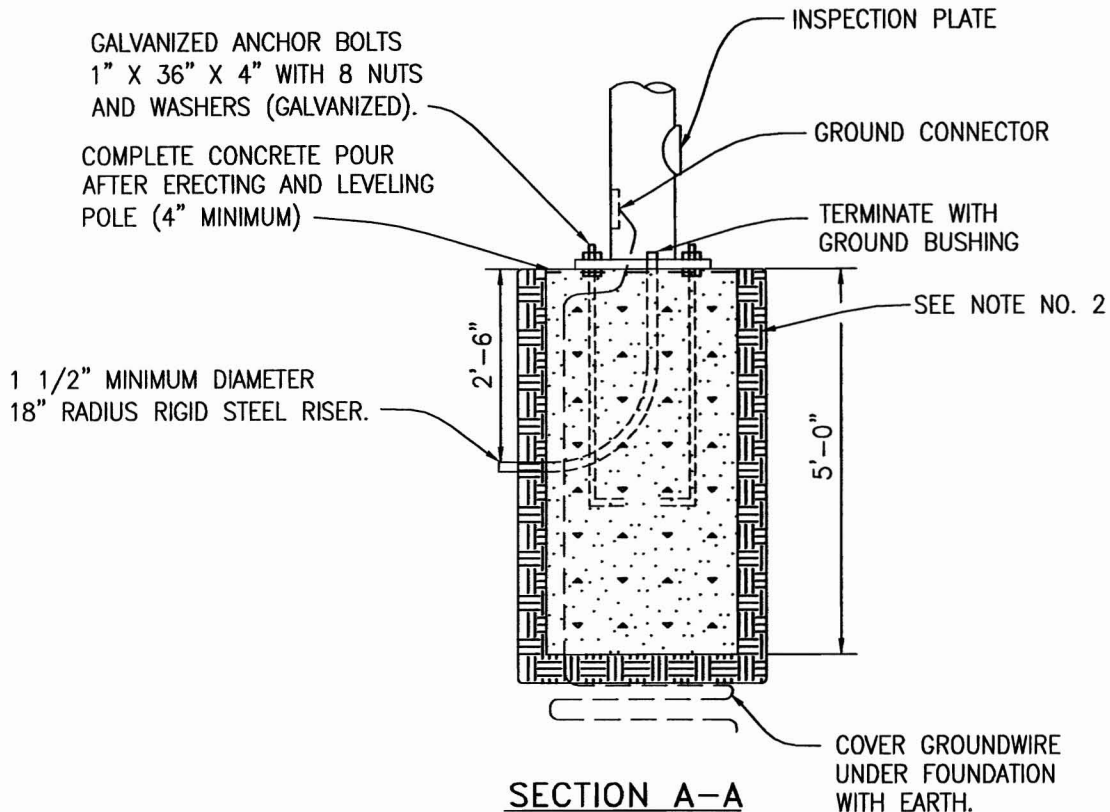
4 - C



NOTE: 30-INCH SQUARE
CAP NOT SHOWN.

MATCH POLE BOLT CIRCLE.
ANCHOR BOLTS (TYP)

PLAN



SECTION A-A

NOTES:

1. GROUND WIRE SHALL BE 15- FEET OF NO. 6 BARE COPPER WIRE. LEAVE 2-FOOT PROTRUDING ABOVE TOP OF FOUNDATION TO ALLOW CONNECTION TO THE STREET LIGHT.
2. IF UNABLE TO POUR FOUNDATION AGAINST UNDISTURBED EARTH, COMPACT TO 90% OF OPTIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D1557.
3. CONCRETE STRENGTH SHALL BE 4000 PSI MINIMUM.
4. PRECAST FOUNDATIONS MAY BE USED WITH CITY ENGINEER APPROVAL.

CITY OF NEWMAN IMPROVEMENT STANDARDS

ELECTROLIER FOUNDATION DETAIL



APPROVED BY:

G. A. [Signature]

DATE: 7/14/09

STD. DETAIL

4 - D



CITY OF NEWMAN IMPROVEMENT STANDARDS

1" DIAMETER WATER SERVICE

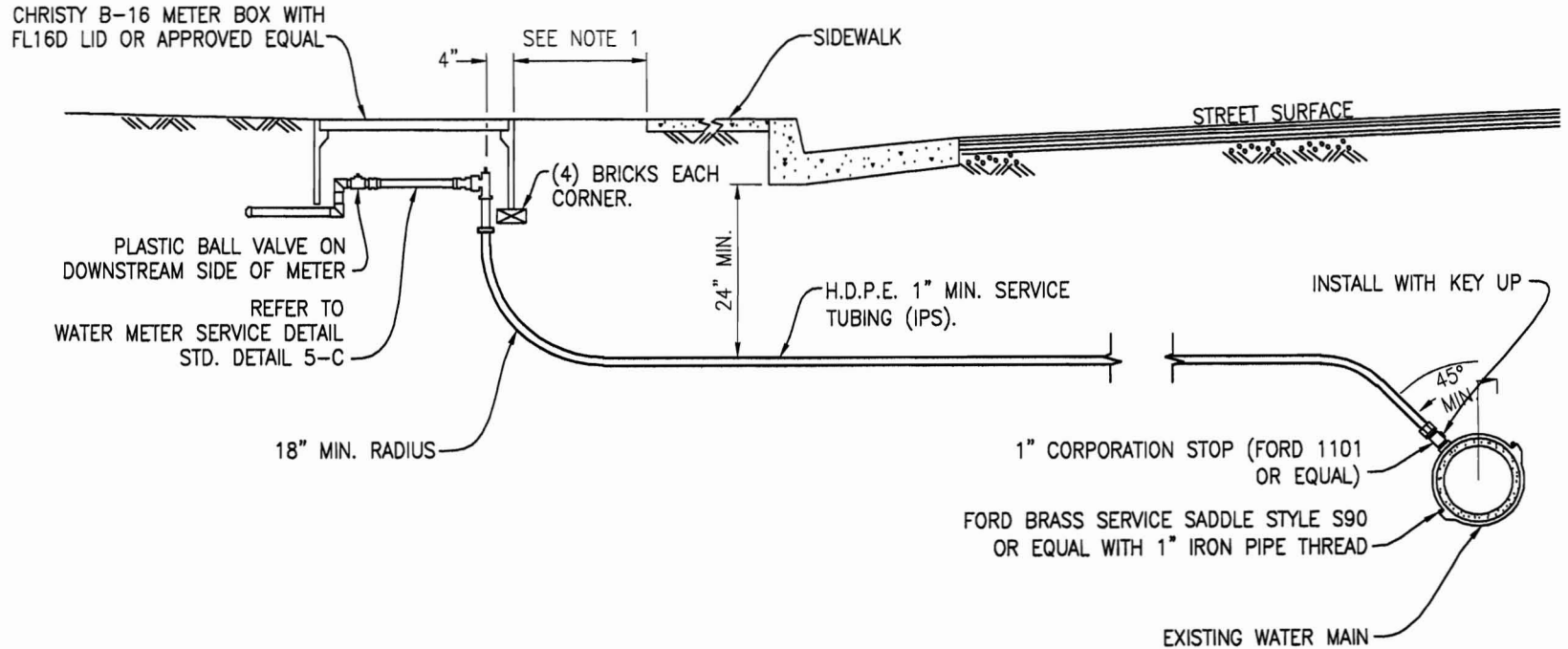
APPROVED BY:

G. L. L. L.

DATE: 7/14/09

STD. DETAIL

5 - A



NOTES:

1. ONE FOOT FROM BACK OF SIDEWALK FOR ATTACHED CURB AND SIDEWALK. FOR AREAS WITH SEPARATED CURB AND SIDEWALK ONE FOOT FROM BACK OF CURB.
2. SERVICE LATERALS SHALL BE POLYETHYLENE PLASTIC PIPE (PE3408, SDR7, PR160 P.S.I.) IN IRON PIPE SIZES AND SHALL BE SUITABLE FOR TRANSPORTING POTABLE WATER. POLYETHYLENE PIPE SHALL COMPLY WITH A.S.T.M. STANDARD D2239.
3. TRAFFIC LID REQUIRED IF LOCATED IN TRAVELED AREA.
4. STAINLESS STEEL INSERT STIFFENERS SHALL BE USED INSIDE P.E. TUBING AT ALL PACK JOINT CONNECTIONS.
5. CURB STOPS SHALL BE DIRECTLY OPPOSITE SERVICE SADDLES UNLESS OTHERWISE APPROVED BY THE CITY ENGINEER.
6. INSTALL TRACER WIRE ALONG SERVICE FROM MAIN TO METER.



CITY OF NEWMAN IMPROVEMENT STANDARDS

2" DIAMETER WATER SERVICE

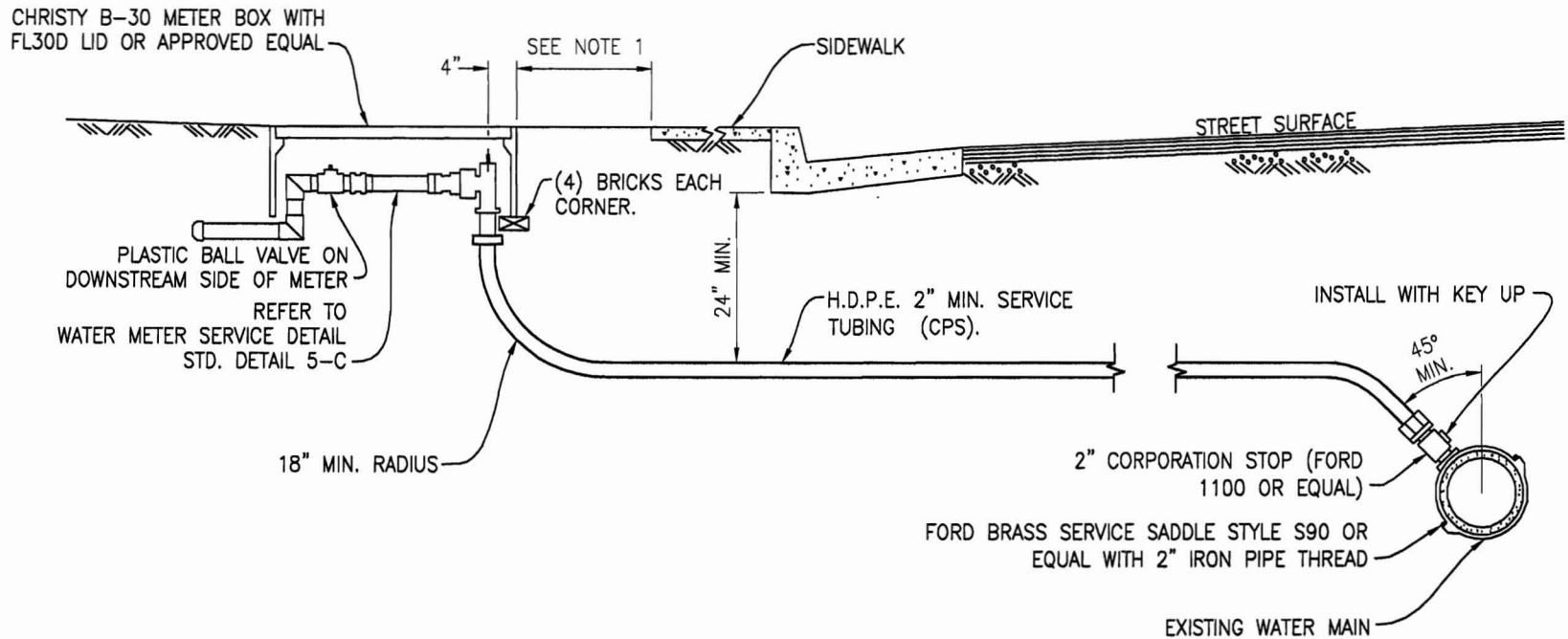
APPROVED BY:

Seal of the City of Newman

DATE: 7/14/09

STD. DETAIL

5-B

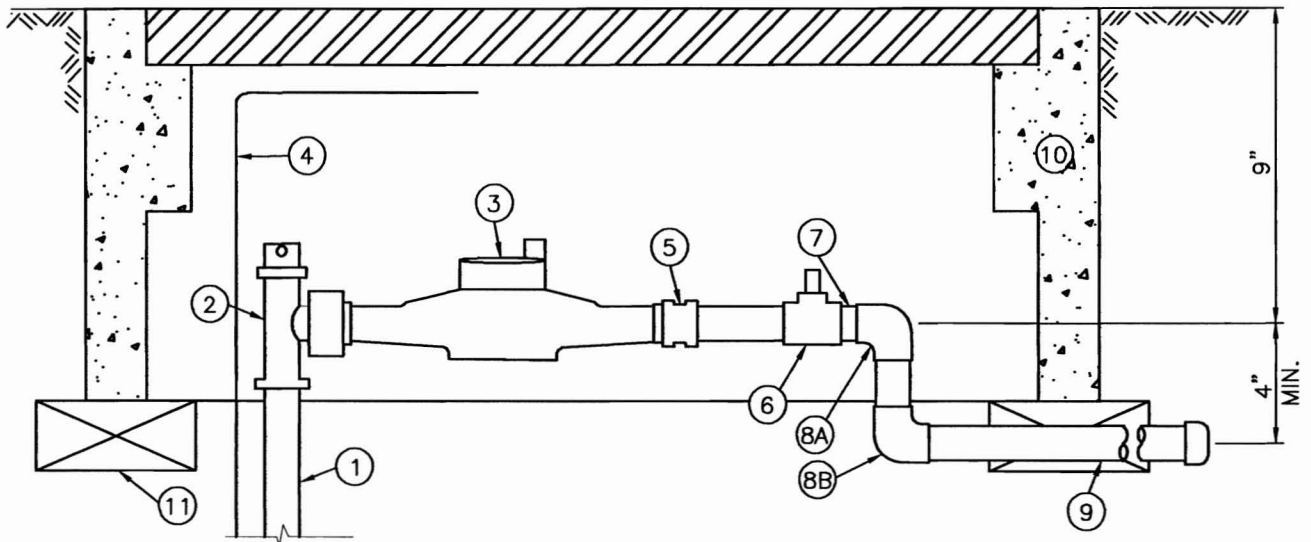


NOTES:

1. ONE FOOT FROM BACK OF SIDEWALK FOR ATTACHED CURB AND SIDEWALK. FOR AREAS WITH SEPARATED CURB AND SIDEWALK ONE FOOT FROM BACK OF CURB.
2. SERVICE LATERALS SHALL BE POLYETHYLENE PLASTIC PIPE (PE3408, SDR7, PR160 P.S.I.) IN COPPER TUBING SIZES AND SHALL BE SUITABLE FOR TRANSPORTING POTABLE WATER. POLYETHYLENE PIPE SHALL COMPLY WITH A.S.T.M. STANDARD D2239.
3. TRAFFIC LID REQUIRED IF LOCATED IN TRAVELED AREA.
4. STAINLESS STEEL INSERT STIFFENERS SHALL BE USED INSIDE P.E. TUBING AT ALL PACK JOINT CONNECTIONS.
5. CURB STOPS SHALL BE DIRECTLY OPPOSITE SERVICE SADDLES UNLESS OTHERWISE APPROVED BY THE CITY ENGINEER.
6. INSTALL TRACER WIRE ALONG SERVICE FROM MAIN TO METER.

NOTES

- ① SERVICE FROM MAIN
- ② ANGLE METER VALVE (FORD BA63 - 444W OR EQUAL FOR 1" SERVICE. FORD BFA43-777W OR EQUAL FOR 2" SERVICE)
- ③ METER - TO BE FURNISHED AND INSTALLED BY THE CITY
- ④ TRACER WIRE
- ⑤ 1" STRAIGHT METER COUPLING (MUELLER H-10896 OR EQUAL) FOR 1" SERVICE. 2" METER FLANGE BY 2" FEMALE IRON PIPE THREAD FOR 2" SERVICE.
- ⑥ PLASTIC BALL VALVE
- ⑦ SCHEDULE 80 THREADED PVC NIPPLE
- ⑧A SCHEDULE 40 THREADED X SLIP PVC ELBOW
- ⑧B SCHEDULE 40 SLIP X SLIP PVC ELBOW
- ⑨ SCHEDULE 40 PVC WITH CAP
- ⑩ METER BOX
- ⑪ BRICK (EACH CORNER OF BOX)



CITY OF NEWMAN IMPROVEMENT STANDARDS

WATER METER SERVICE DETAIL



APPROVED BY: *G. R. Gul*

DATE: 7/14/09

STD. DETAIL

5 - C



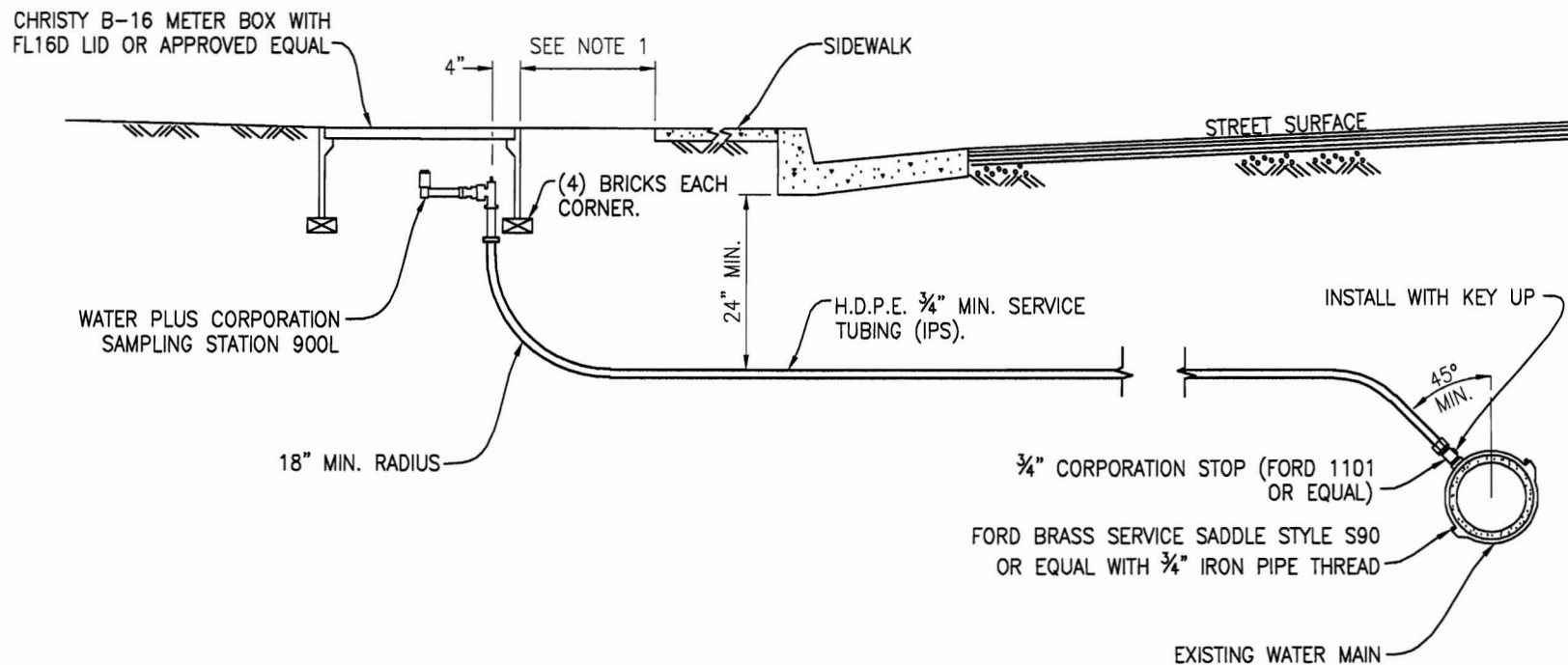
CITY OF NEWMAN IMPROVEMENT STANDARDS

SAMPLING STATION

APPROVED BY: *Carla Regal* DATE: 7/14/09

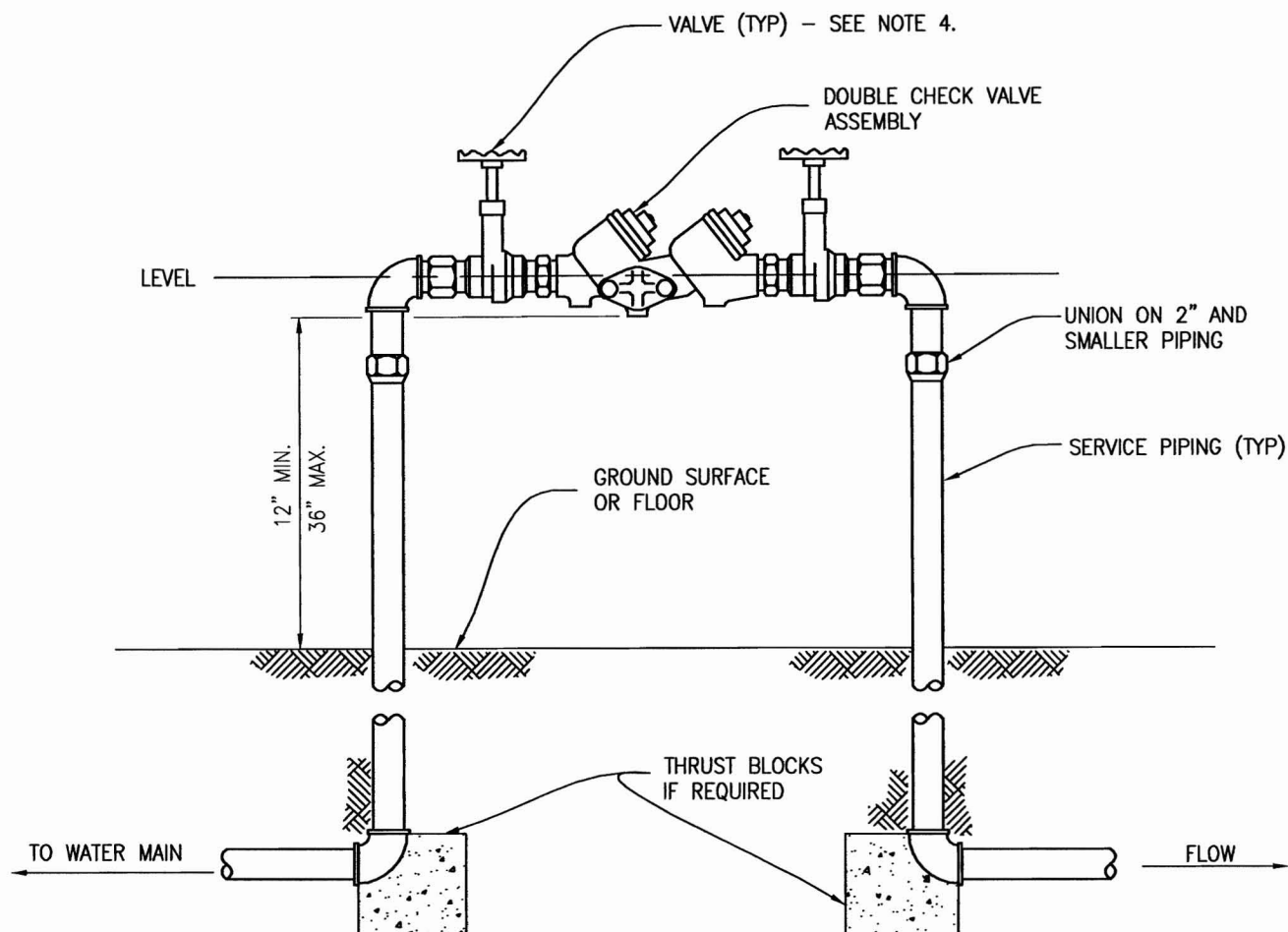
STD. DETAIL

5 - D



NOTES:

1. ONE FOOT FROM BACK OF SIDEWALK FOR ATTACHED CURB AND SIDEWALK. FOR AREAS WITH SEPARATED CURB AND SIDEWALK ONE FOOT FROM BACK OF CURB.
2. SERVICE LATERALS SHALL BE POLYETHYLENE PLASTIC PIPE (PE3408, SDR7, PR160 P.S.I.) IN IRON PIPE SIZES AND SHALL BE SUITABLE FOR TRANSPORTING POTABLE WATER. POLYETHYLENE PIPE SHALL COMPLY WITH A.S.T.M. STANDARD D2239.
3. TRAFFIC LID REQUIRED IF LOCATED IN TRAVELED AREA.
4. STAINLESS STEEL INSERT STIFFENERS SHALL BE USED INSIDE P.E. TUBING AT ALL PACK JOINT CONNECTIONS.
5. CURB STOPS SHALL BE DIRECTLY OPPOSITE SERVICE SADDLES UNLESS OTHERWISE APPROVED BY THE CITY ENGINEER.
6. INSTALL TRACER WIRE ALONG SERVICE FROM MAIN TO SAMPLING STATION.



NOTES:

1. MANUFACTURING AND MODEL NUMBER OF BACKFLOW PREVENTION ASSEMBLY SHALL BE APPROVED BY THE DIRECTOR PRIOR TO INSTALLATION.
2. ABOVE GROUND SERVICE PIPING FOR BACKFLOW PREVENTION DEVICES 2 INCHES IN DIAMETER AND SMALLER SHALL BE TYPE K COPPER OR BRASS. SERVICE LINES 4-INCH DIAMETER AND SMALLER BUT LARGER THAN 2-INCH IN DIAMETER SHALL BE GALVANIZED STEEL. WRAP ALL METALLIC PIPING BELOW GROUND AND WITHIN 6 INCHES OF GROUND SURFACE WITH 2 LAYERS OF 10-MIL CALPICO TAPE.
3. SUPPORTS WHERE PROVIDED SHALL NOT INTERFERE WITH TESTING AND MAINTENANCE.
4. BALL VALVES SHALL BE INSTALLED ON 2-INCH AND SMALLER BACKFLOW PREVENTION ASSEMBLIES. GATE VALVES SHALL BE UTILIZED ON BACKFLOW PREVENTION ASSEMBLIES LARGER THAN 2-INCH.
5. INSTALL AN INSULATING BAG AND EXPANDED METAL CAGE AS APPROVED BY DIRECTOR AROUND ASSEMBLY.
6. BACKFLOW PREVENTION ASSEMBLY SHOWN IS FOR NON-FIRE FLOW APPLICATION. BACKFLOW PREVENTION INSTALLATIONS FOR FIRE PREVENTION SHALL BE AS APPROVED BY THE FIRE CHIEF.

CITY OF NEWMAN IMPROVEMENT STANDARDS



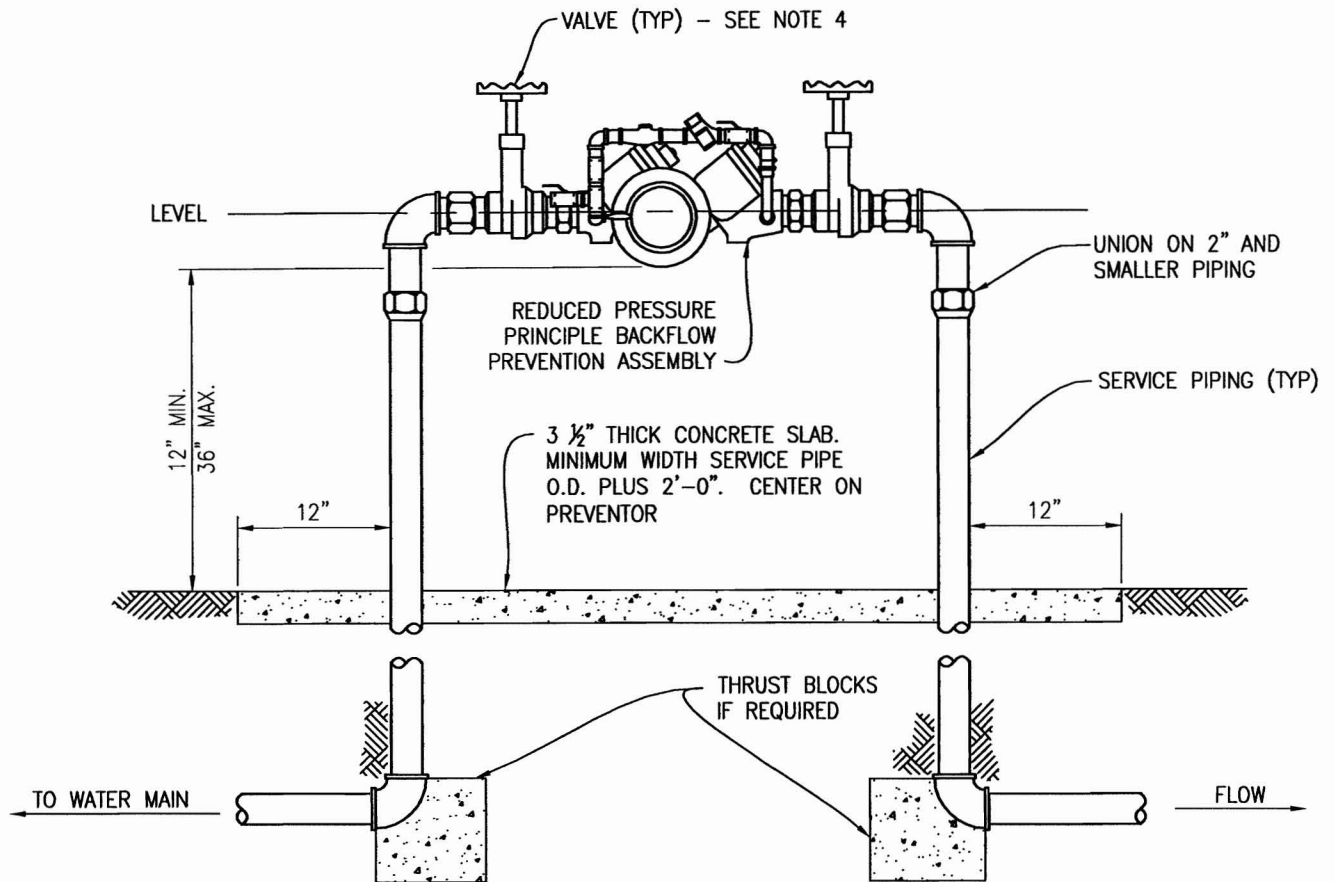
DOUBLE CHECK VALVE BACKFLOW PREVENTER

APPROVED BY:

G. K. [Signature] DATE: 7/14/09

STD. DETAIL

5 - E



NOTES:

1. MANUFACTURING AND MODEL NUMBER OF BACKFLOW PREVENTION ASSEMBLY SHALL BE APPROVED BY THE DIRECTOR PRIOR TO INSTALLATION.
2. ABOVE GROUND SERVICE PIPING FOR BACKFLOW PREVENTION DEVICES 2 INCHES IN DIAMETER AND SMALLER SHALL BE TYPE K COPPER OR BRASS. SERVICE LINES 4-INCH DIAMETER AND SMALLER BUT LARGER THAN 2-INCH IN DIAMETER SHALL BE GALVANIZED STEEL. WRAP ALL METALLIC PIPING BELOW GROUND AND WITHIN 6 INCHES OF GROUND SURFACE WITH 2 LAYERS OF 10-MIL CALPICO TAPE.
3. SUPPORTS WHERE PROVIDED SHALL NOT INTERFERE WITH TESTING AND MAINTENANCE.
4. BALL VALVES SHALL BE INSTALLED ON 2-INCH AND SMALLER BACKFLOW PREVENTION ASSEMBLIES. GATE VALVES SHALL BE UTILIZED ON BACKFLOW PREVENTION ASSEMBLIES LARGER THAN 2-INCH.
5. INSTALL AN INSULATING BAG AND EXPANDED METAL CAGE AS APPROVED BY DIRECTOR AROUND ASSEMBLY.

CITY OF NEWMAN IMPROVEMENT STANDARDS



REDUCED PRESSURE BACKFLOW PREVENTER

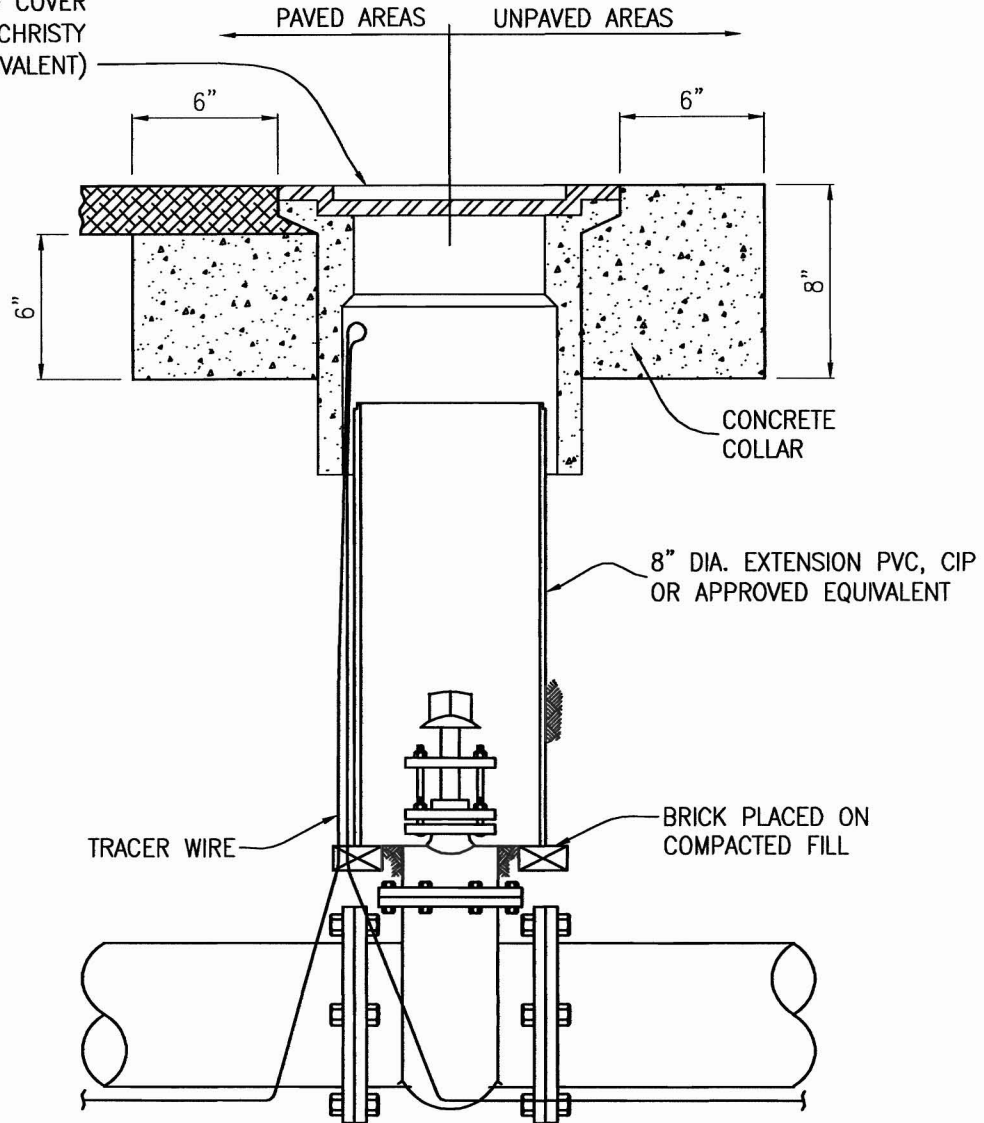
APPROVED BY: *Eric Lohr*

DATE: 7/14/09

STD. DETAIL

5 - F

VALVE BOX WITH CAST
IRON FACE AND COVER
MARKED WATER (CHRISTY
G-5, OR EQUIVALENT)



CITY OF NEWMAN IMPROVEMENT STANDARDS



VALVE BOX DETAIL

APPROVED BY:

Gia R. G. G.

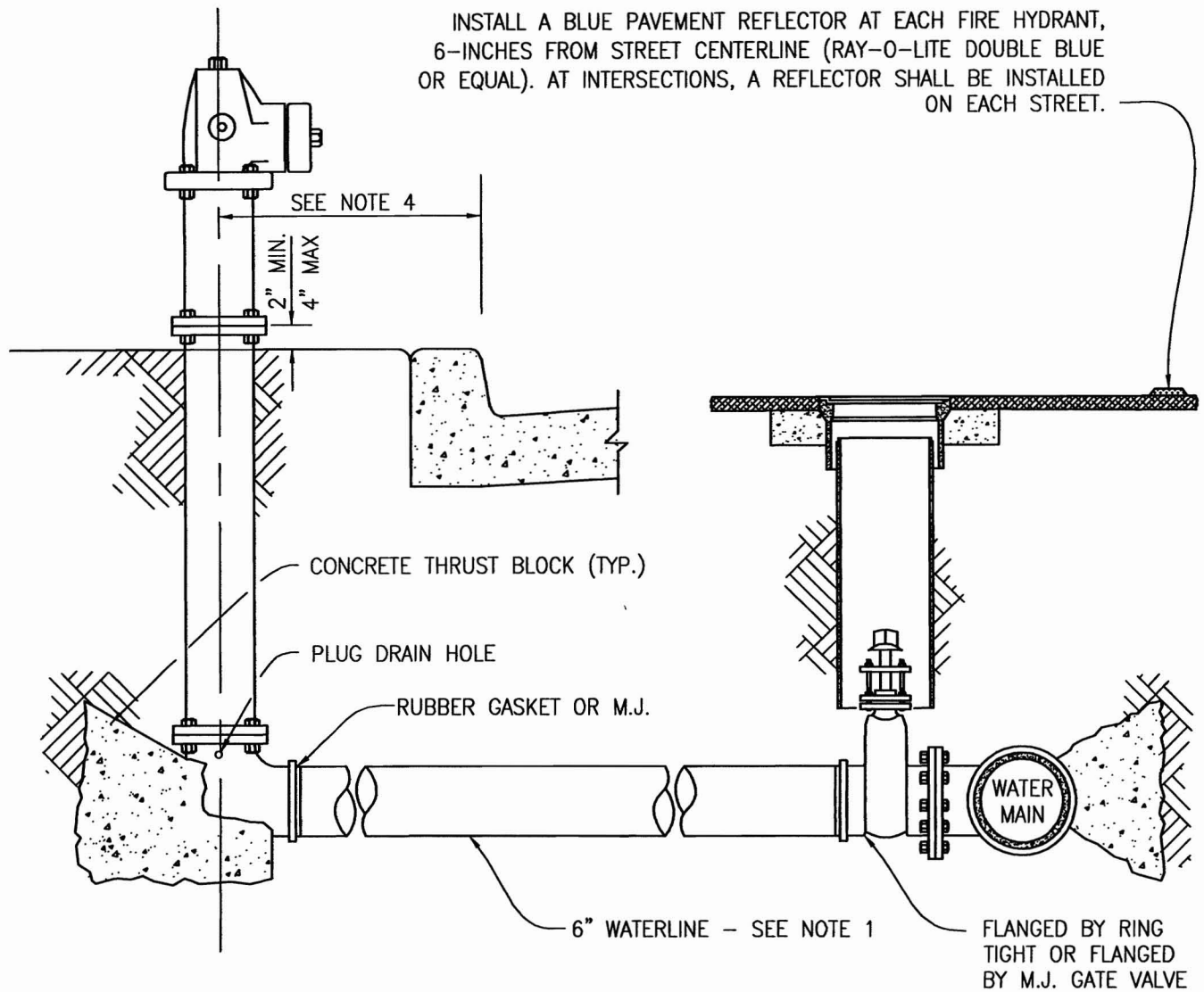
DATE:

7/14/09

STD. DETAIL

5 - G

INSTALL A BLUE PAVEMENT REFLECTOR AT EACH FIRE HYDRANT, 6-INCHES FROM STREET CENTERLINE (RAY-O-LITE DOUBLE BLUE OR EQUAL). AT INTERSECTIONS, A REFLECTOR SHALL BE INSTALLED ON EACH STREET.



NOTES:

1. HYDRANT RUN BETWEEN GATE VALVE AND HYDRANT ELBOW SHALL NOT RISE MORE THAN 1/4-INCH PER FOOT.
2. WRAP THE FIRE HYDRANT AND WATER MAIN FITTINGS WITH 6-MIL PLASTIC PRIOR TO POURING THRUST BLOCKS.
3. SEE STD. DETAIL 5-G FOR VALVE BOX DETAILS.
4. DISTANCE FROM FACE OF CURB TO CENTERLINE OF HYDRANT SHALL BE 2'-6" WHERE HYDRANT IS LOCATED WITHIN A PARK STRIP. WHERE SIDEWALK IS LOCATED ADJACENT TO THE SIDEWALK, THE CENTERLINE OF THE HYDRANT SHALL BE LOCATED 2'-0" BEHIND THE SIDEWALK.

CITY OF NEWMAN IMPROVEMENT STANDARDS



FIRE HYDRANT INSTALLATION

APPROVED BY:

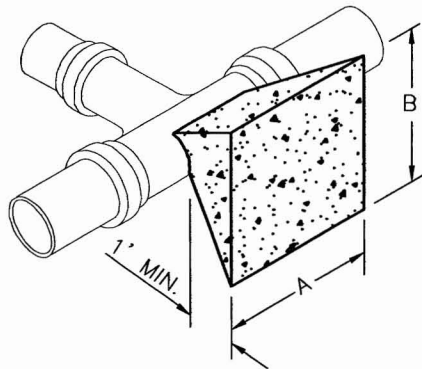
G. K. [Signature]

DATE:

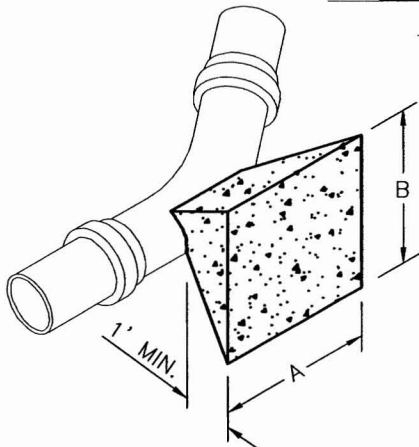
7/14/09

STD. DETAIL

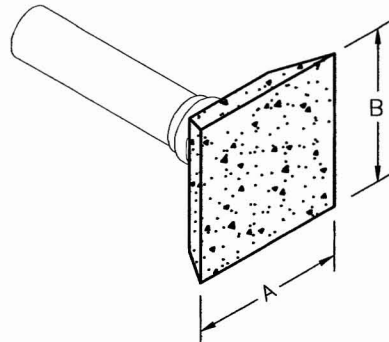
5 - H



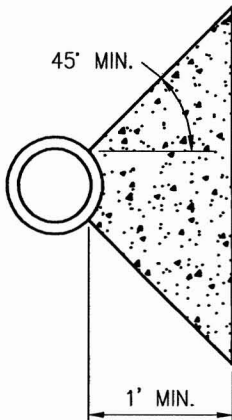
TYPICAL THRUST BLOCK
TEE OUTLET



TYPICAL THRUST BLOCK
CAST IRON BEND



TYPICAL THRUST BLOCK
DEAD END



TYPICAL SECTION THROUGH
THRUST BLOCK

THRUST BLOCK AREA REQUIRED

FITTINGS	ALLOWABLE SOIL BEARING VALUE	
	1,500 LBS. PER SQ. FT.	
6" LINE	"A"	"B"
22-1/2'	1'-6"	1'-0"
45'	2'-0"	1'-3"
90'	3'-0"	1'-6"
TEE OUTLET	2'-0"	1'-6"
DEAD END	2'-0"	1'-6"
8" LINE	"A"	"B"
22-1/2'	1'-6"	1'-6"
45'	2'-6"	1'-6"
90'	4'-0"	2'-0"
TEE OUTLET	2'-9"	2'-0"
DEAD END	2'-9"	2'-0"
10" LINE	"A"	"B"
22-1/2'	2'-3"	1'-6"
45'	3'-0"	2'-0"
90'	5'-0"	2'-6"
TEE OUTLET	3'-3"	2'-6"
DEAD END	3'-3"	2'-6"
12" LINE	"A"	"B"
22-1/2'	2'-6"	2'-0"
45'	4'-0"	2'-6"
90'	6'-0"	3'-0"
TEE OUTLET	4'-0"	3'-0"
DEAD END	4'-0"	3'-0"

NOTE:

1. ALL VALUES SHOWN ARE MINIMUM FOR A HYDROSTATIC PRESSURE OF 150 P.S.I. AND A SOIL RESISTANCE OF 1,500 LBS./SQ. FT. WITH A MINIMUM PIPELINE COVER OF 2'-6".
2. REDUCTION OF THE THRUST BLOCK SURFACE AREA WILL BE CONSIDERED UPON SUBMITTAL OF APPROVED SOIL BEARING TEST RESULTS GREATER THAN 1,500 LBS./SQ. FT.
3. ALL THRUST BLOCKS SHALL BE POURED AGAINST UNDISTURBED SOIL.
4. THRUST BLOCKS SHOWN ARE FOR HORIZONTAL THRUST RESTRAINT. FOR VERTICAL THRUST RESTRAINT, DETAILS SHALL BE PROVIDED BY DESIGN ENGINEER.

CITY OF NEWMAN IMPROVEMENT STANDARDS



THRUST BLOCK DETAILS

APPROVED BY:

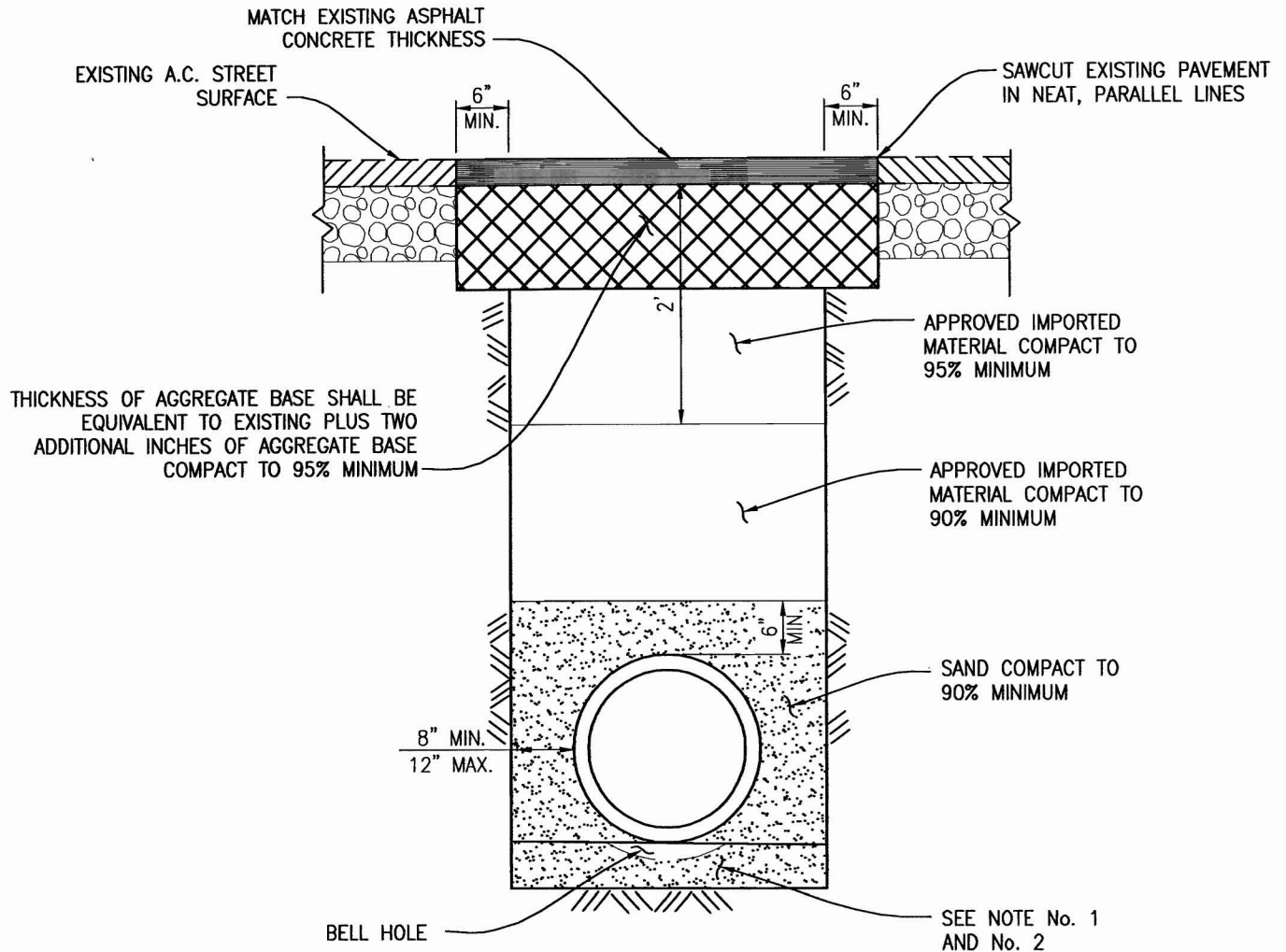
Ga. P. G. G.

DATE:

7/14/09

STD. DETAIL

5 - 1



NOTES:

1. IF BOTTOM OF TRENCH IS SOFT OR UNSTABLE, IT SHALL BE OVEREXCAVATED A MINIMUM OF 1. FOOT BELOW GRADE AND BACKFILLED WITH APPROVED IMPORTED MATERIAL.
2. FOR PVC PRESSURE PIPE PROVIDE 4 INCHES OF SAND BEDDING, COMPACT TO 90% MINIMUM.
3. SAND SHALL CONFORM TO THE FOLLOWING GRADING REQUIREMENTS:

SIEVE SIZE	PERCENTAGE PASSING
#4	100
#16	0-70
#30	0-15
#200	0-5

4. TRENCHES NOT IN PAVED AREAS SHALL BE RESTORED TO MATCH EXISTING SURFACE CONDITIONS.
5. SOIL DENSITIES ARE EXPRESSED AS A PERCENTAGE OF MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D 1557.

CITY OF NEWMAN IMPROVEMENT STANDARDS



WATER LINE TRENCH DETAIL

APPROVED BY:

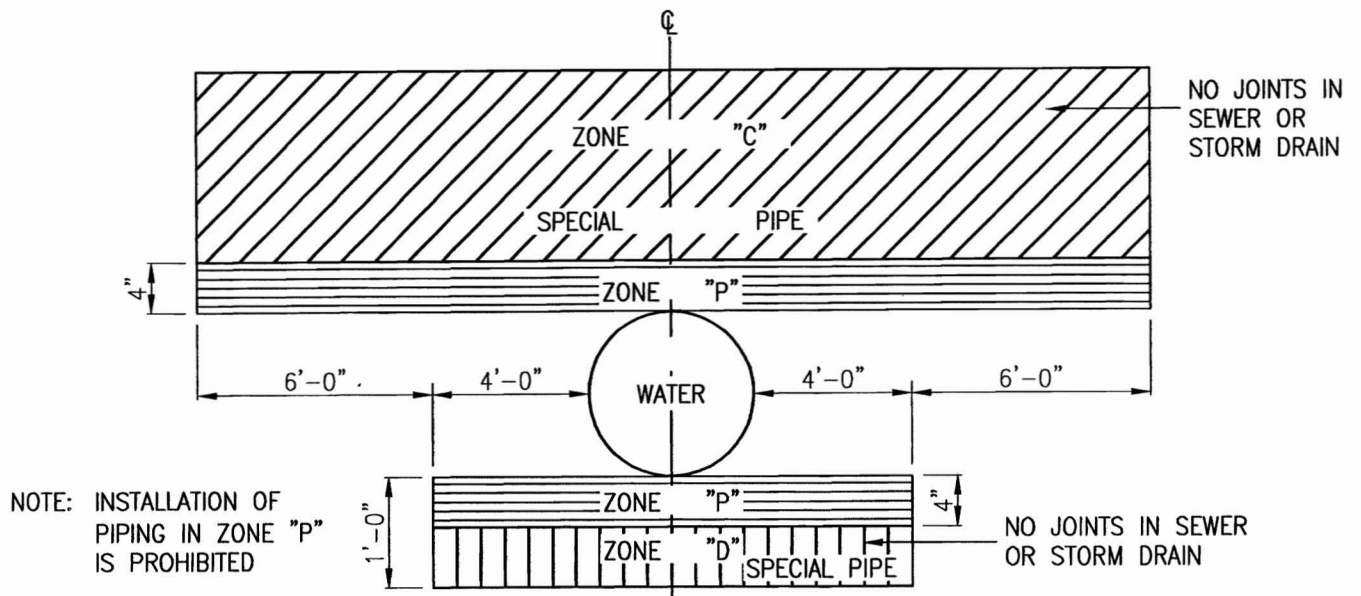
Garland

DATE:

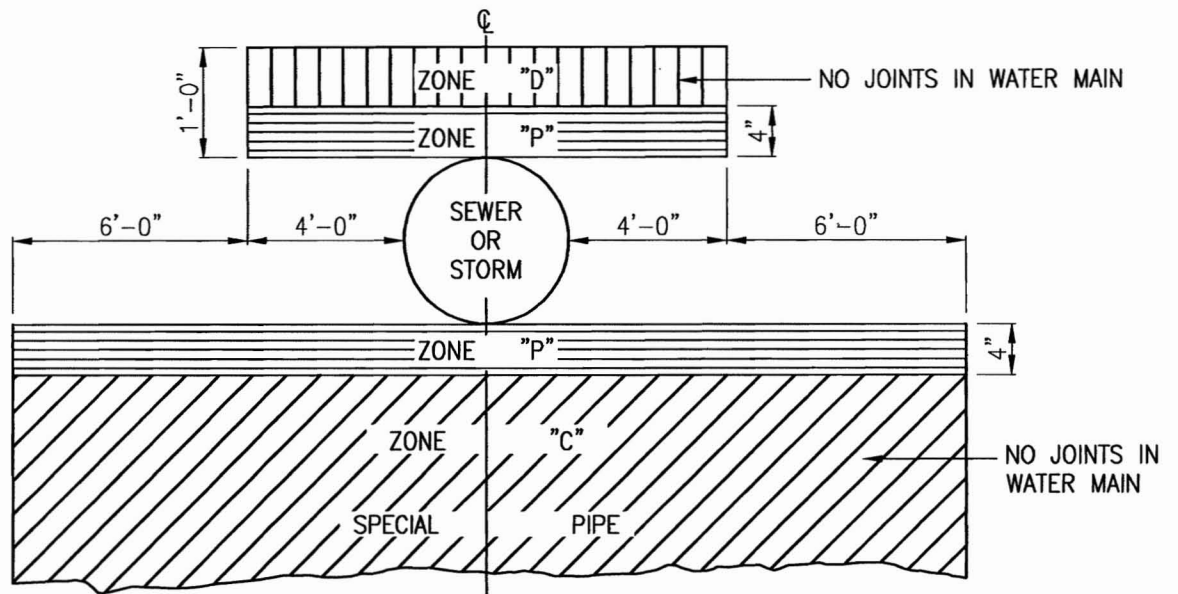
7/14/09

STD. DETAIL

5 - J



EXISTING WATERLINE - NEW SANITARY SEWER OR STORM DRAIN



EXISTING SEWER LINE OR STORM DRAIN - NEW WATERLINE

CITY OF NEWMAN IMPROVEMENT STANDARDS

CROSSINGS



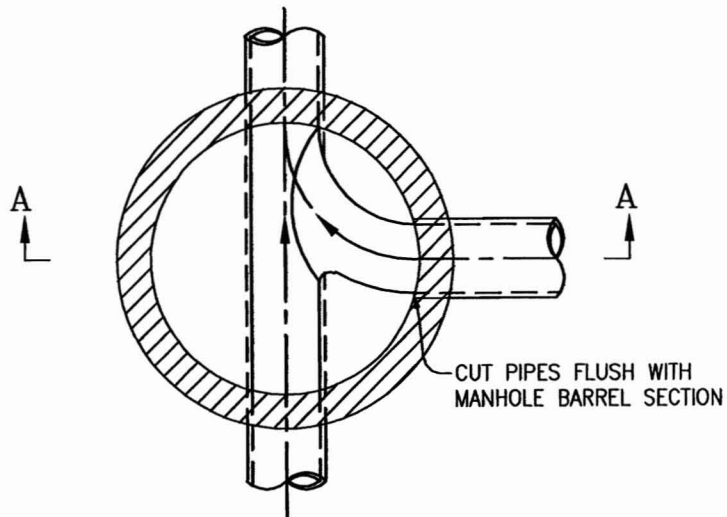
APPROVED BY:

Ga. K. G. G.

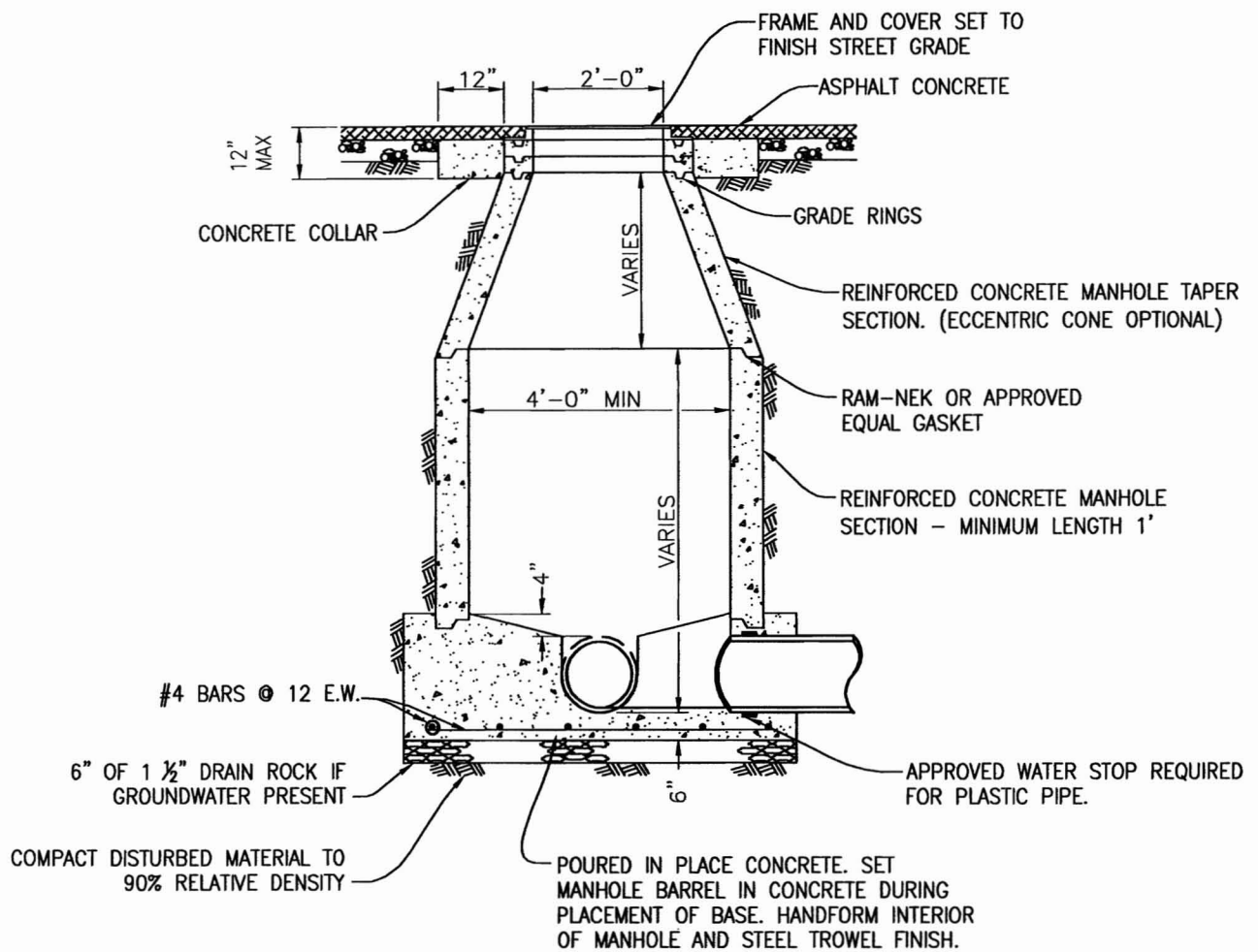
DATE: 7/14/09

STD. DETAIL

5 - K



PLAN



SECTION "A-A"

CITY OF NEWMAN IMPROVEMENT STANDARDS

SANITARY SEWER MANHOLE



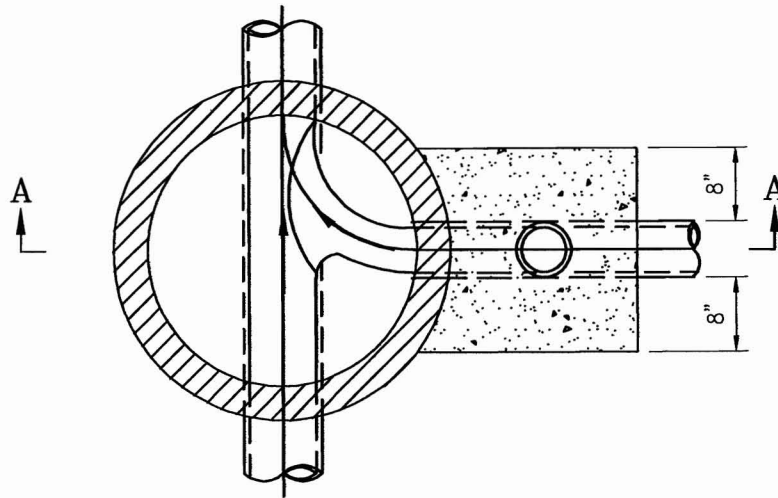
APPROVED BY:

G. R. R. R.

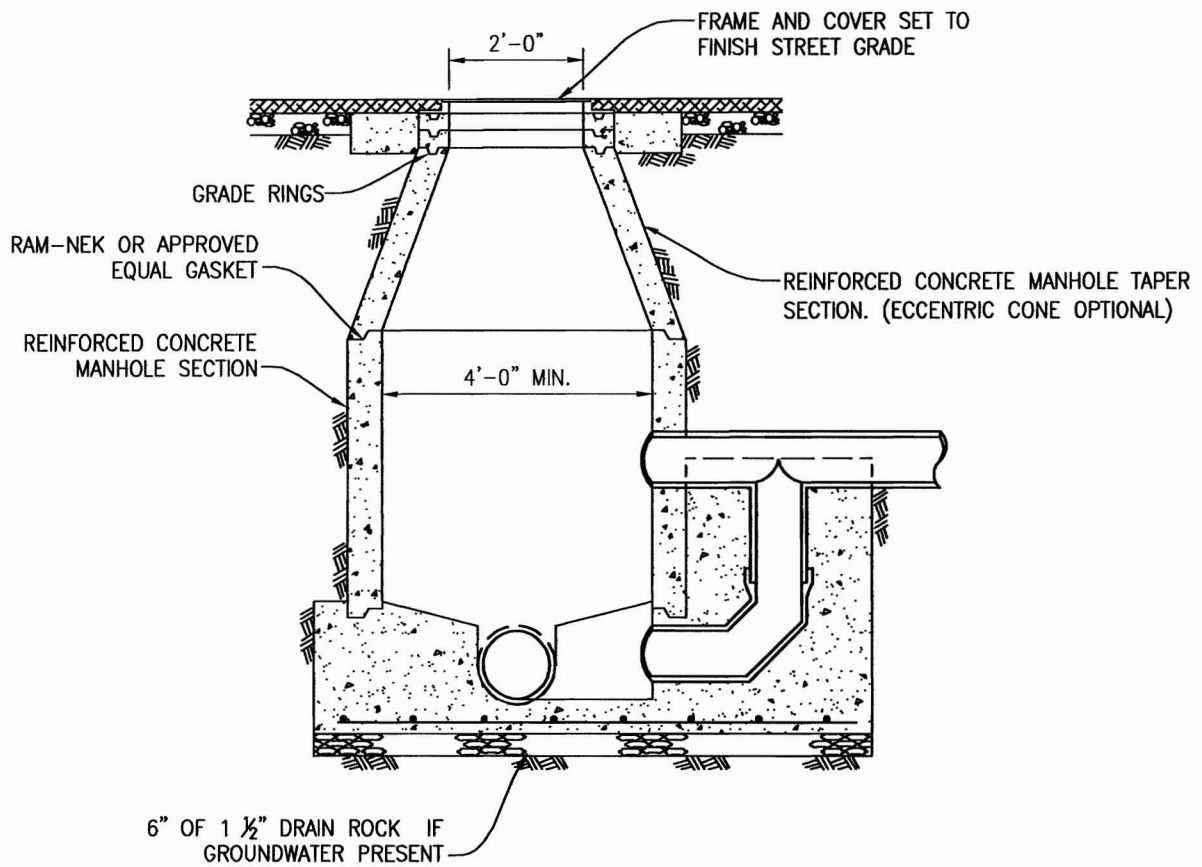
DATE: 7/14/09

STD. DETAIL

6 - A



PLAN



SECTION "A-A"

NOTE:
SEE STD. DETAIL 6-A FOR
ADDITIONAL INFORMATION

CITY OF NEWMAN IMPROVEMENT STANDARDS

SANITARY SEWER DROP MANHOLE

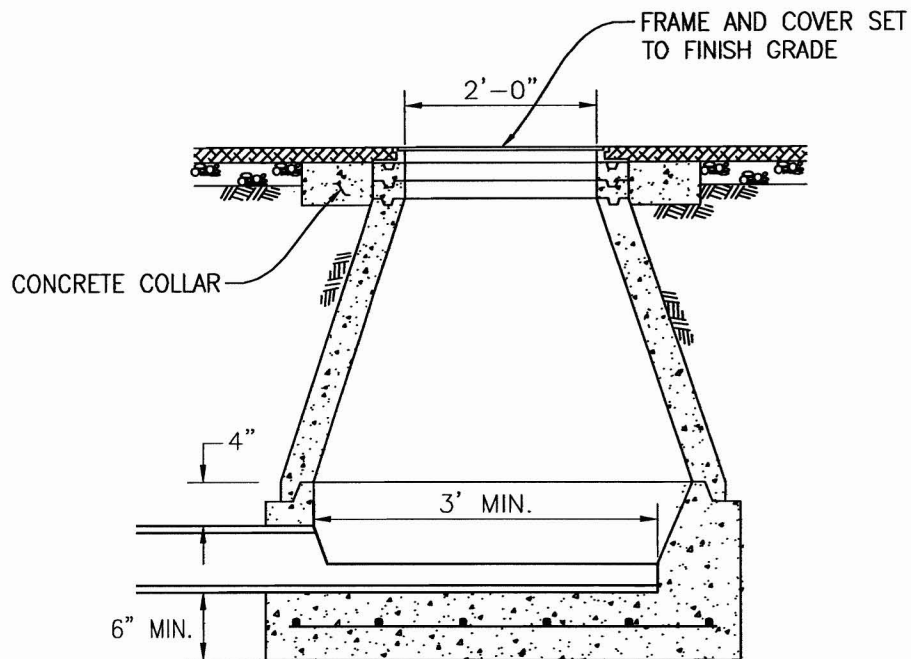


APPROVED BY: *Garland*

DATE: 7/14/09

STD. DETAIL

6 - B



MANHOLE SECTION

NOTE:

SEE STD. DETAIL 6-A FOR
ADDITIONAL INFORMATION

CITY OF NEWMAN IMPROVEMENT STANDARDS



TERMINAL MANHOLE

APPROVED BY

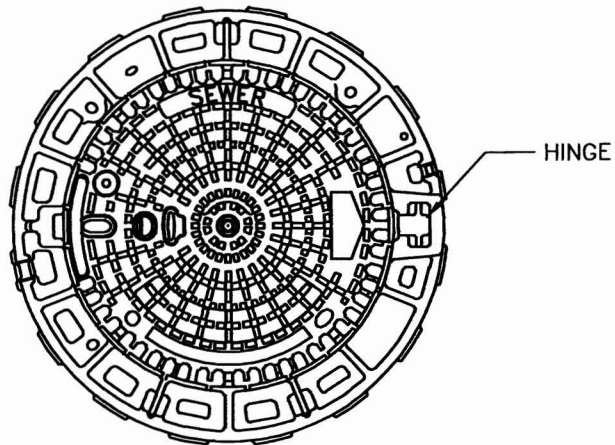
G. Adkins

DATE:

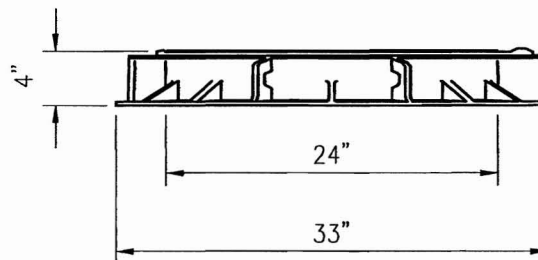
7/14/09

STD. DETAIL

6 - C



PLAN



ELEVATION

NOTES:

1. CERTAINTEED PAMREX MANHOLE FRAME AND COVER SHALL BE UTILIZED.
MINIMUM WEIGHT OF DUCTILE IRON FRAME AND COVER 195 LBS.

CITY OF NEWMAN IMPROVEMENT STANDARDS



PAMREX MANHOLE FRAME AND COVER

APPROVED BY:

Eric R. [Signature]

DATE:

7/14/09

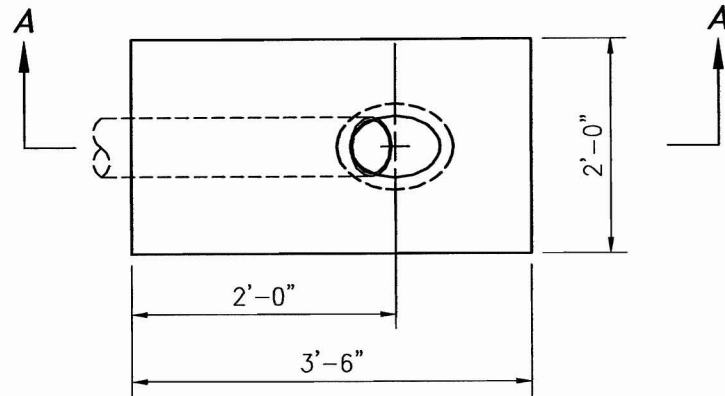
STD. DETAIL

6 - D

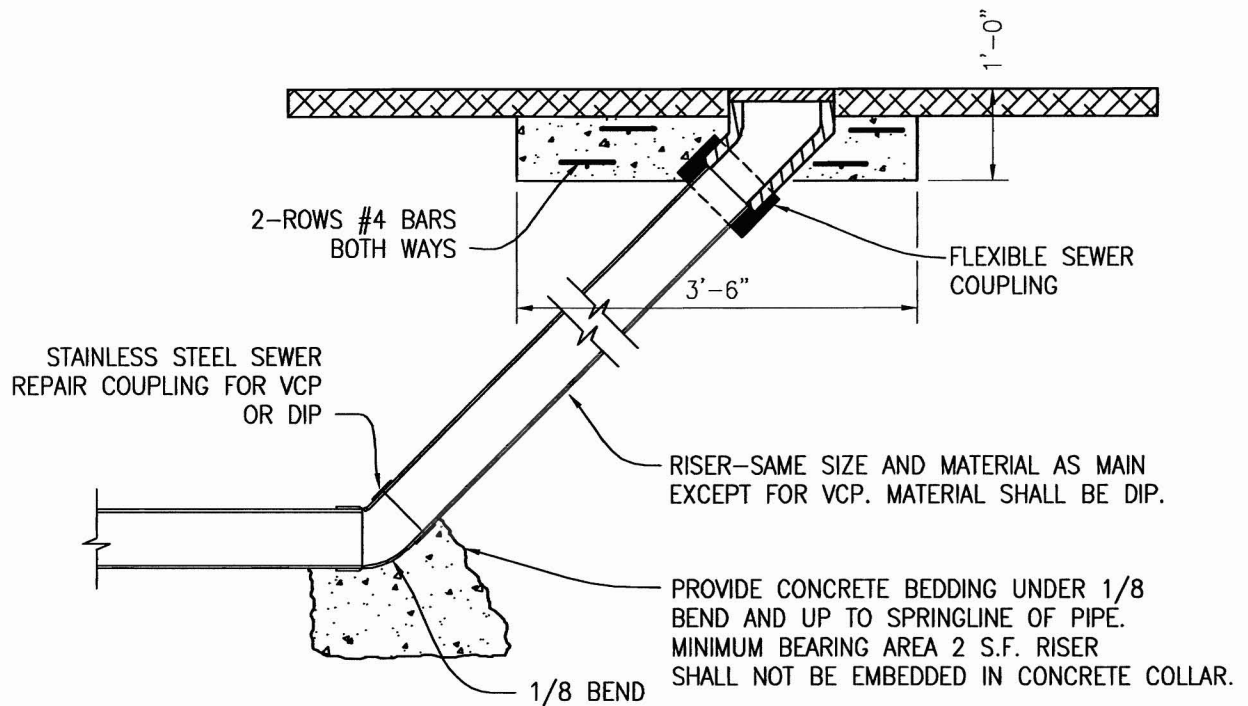
CASTING FRAME AND COVER

6" SOUTH BAY FOUNDRY
NO. SBF 1248 OR EQUAL.

8" SOUTH BAY FOUNDRY
NO. SBF 1247 OR EQUAL.



PLAN



SECTION "A-A"

CITY OF NEWMAN IMPROVEMENT STANDARDS

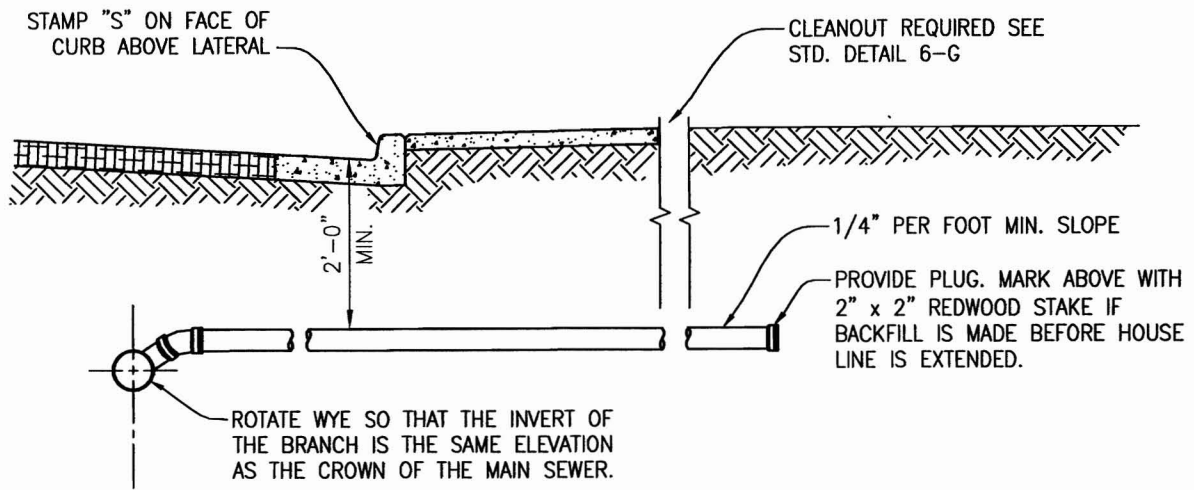
SANITARY SEWER CLEANOUT



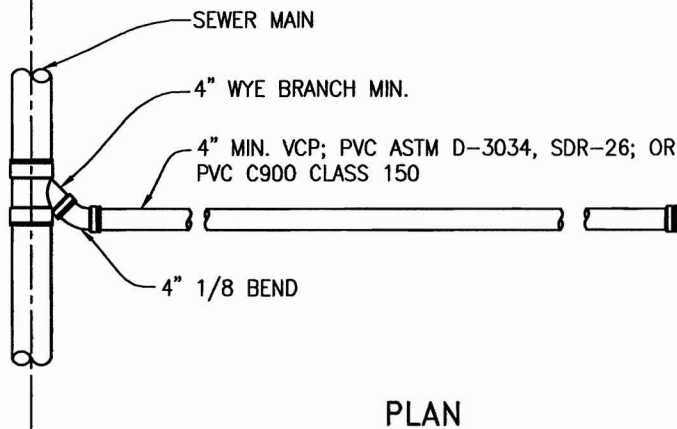
APPROVED BY: *Ga. B. Ryzak* DATE: 7/14/09

STD. DETAIL

6 - E



PROFILE



PLAN

NOTES:

1. LATERALS SHALL HAVE SAME BEDDING & BACKFILL AS SEWER MAIN. TRENCH WIDTH FOR 4-INCH OR SMALLER LINES SHALL BE SUFFICIENT TO ALLOW MECHANICAL COMPACTION OF BACKFILL AROUND CONDUIT.
2. SEWER SERVICE SHALL HAVE A MINIMUM OF 2 FEET COVER AT PROPERTY LINE WHENEVER LATERAL DEPTH AND SERVICE SLOPE OF 1/4 INCH PER FOOT (MIN.) PERMIT. SERVICE SHALL BE DEEP ENOUGH TO SERVE ADJACENT PROPERTY.
3. WHEN THE LATERAL SEWER DEPTH IS SUCH THAT 2 FEET COVER AT PROPERTY LINE CANNOT BE MET, THE LATERAL MUST BE ENCASED IN CONCRETE IN THE TRAVELED RIGHT OF WAY OR PVC PIPE MEETING THE REQUIREMENTS OF AWWA C900 CLASS 150 SHALL BE USED.

CITY OF NEWMAN IMPROVEMENT STANDARDS



4-INCH SEWER SERVICE

APPROVED BY:

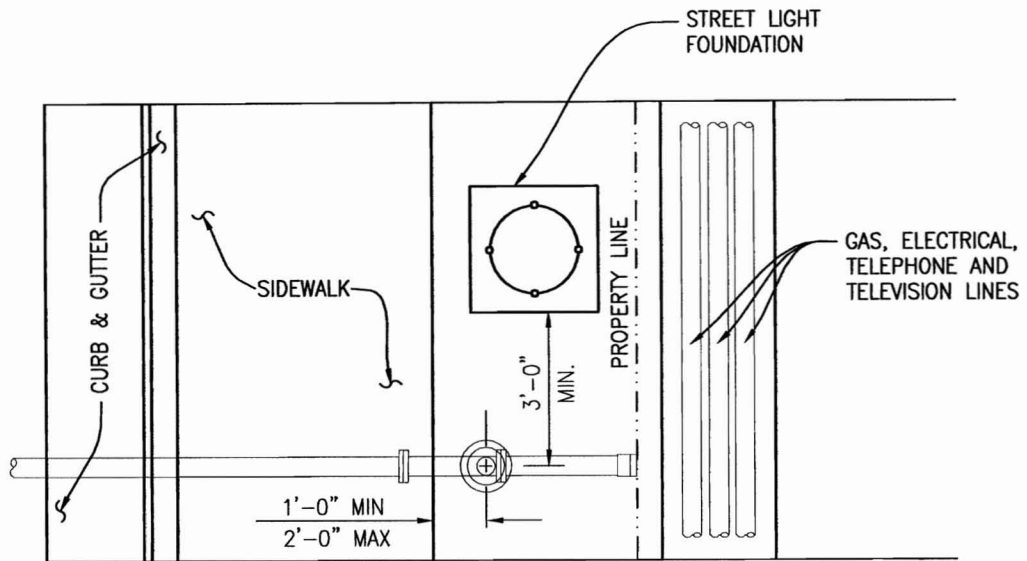
G. P. [Signature]

DATE:

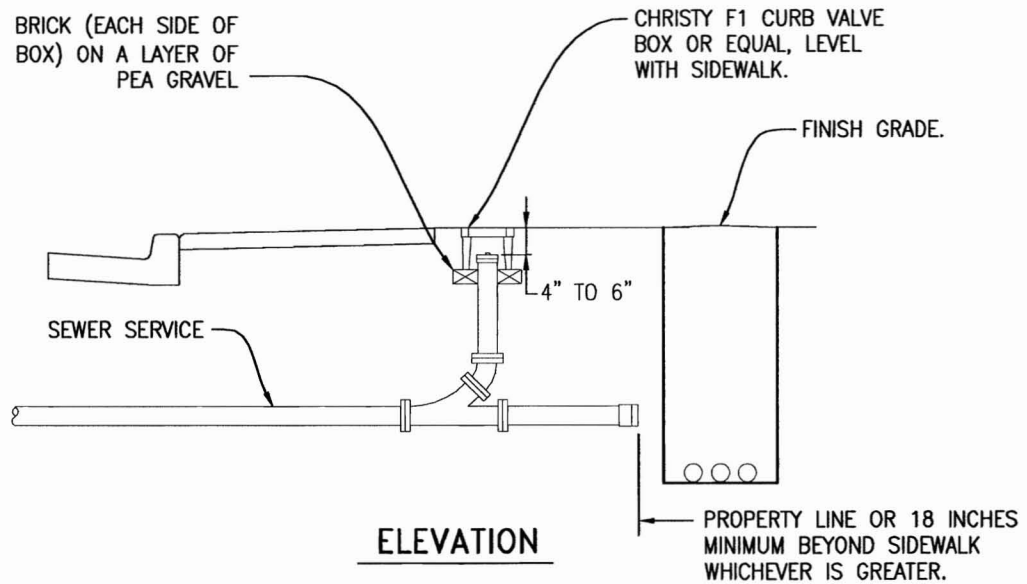
7/14/09

STD. DETAIL

6 - F



PLAN



ELEVATION

NOTES:

1. THE RISER SHALL BE TERMINATED APPROXIMATELY 6 INCHES BELOW GRADE WITH A SOLVENT WELD CAP. THE CLEANOUT CAP, PLUG AND CURB VALVE BOX SHALL BE INSTALLED WHEN CONNECTION TO THE SEWER SERVICE IS MADE.
2. CLEANOUT LOCATION IS SHOWN BASED ON ATTACHED CURB AND SIDEWALK. FOR SEPARATED CURB AND SIDEWALK, INSTALL CLEANOUT 2 FEET BEHIND CURB.

CITY OF NEWMAN IMPROVEMENT STANDARDS

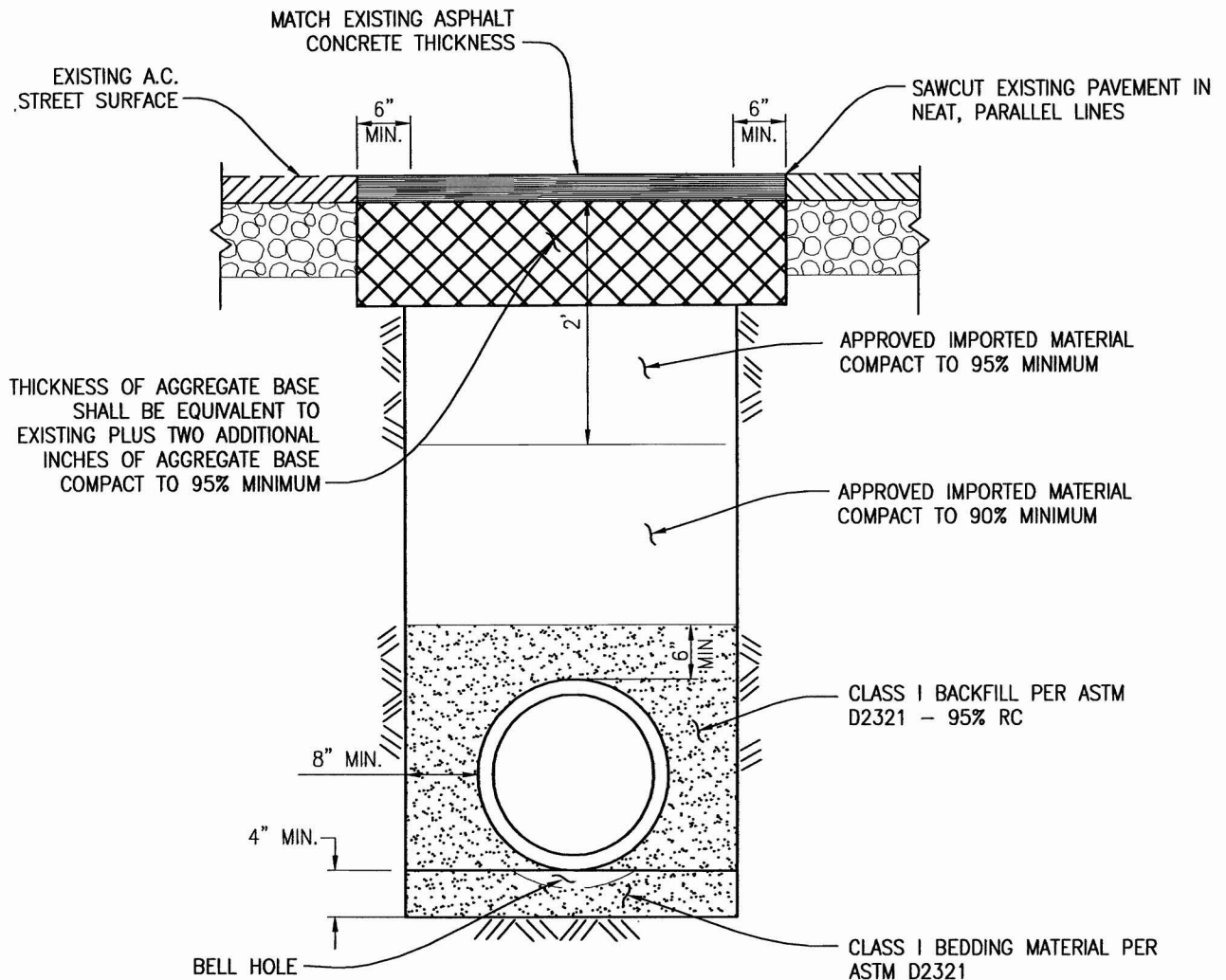


4-INCH SERVICE CLEANOUT

APPROVED BY: *Gra Belynd* DATE: *7/14/09*

STD. DETAIL

6 - G



NOTES:

1. IF THE BOTTOM OF TRENCH IS SOFT OR UNSTABLE, IT SHALL BE OVER-EXCAVATED 1 FOOT BELOW GRADE AND BACKFILLED WITH APPROVED IMPORTED MATERIAL.
2. TRENCHES NOT IN PAVED AREAS SHALL BE RESTORED TO MATCH EXISTING SURFACE CONDITIONS.
3. SOIL DENSITIES ARE EXPRESSED AS A PERCENTAGE OF MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D 1557.

CITY OF NEWMAN IMPROVEMENT STANDARDS



FLEXIBLE WALL PIPE BACKFILL

APPROVED BY:

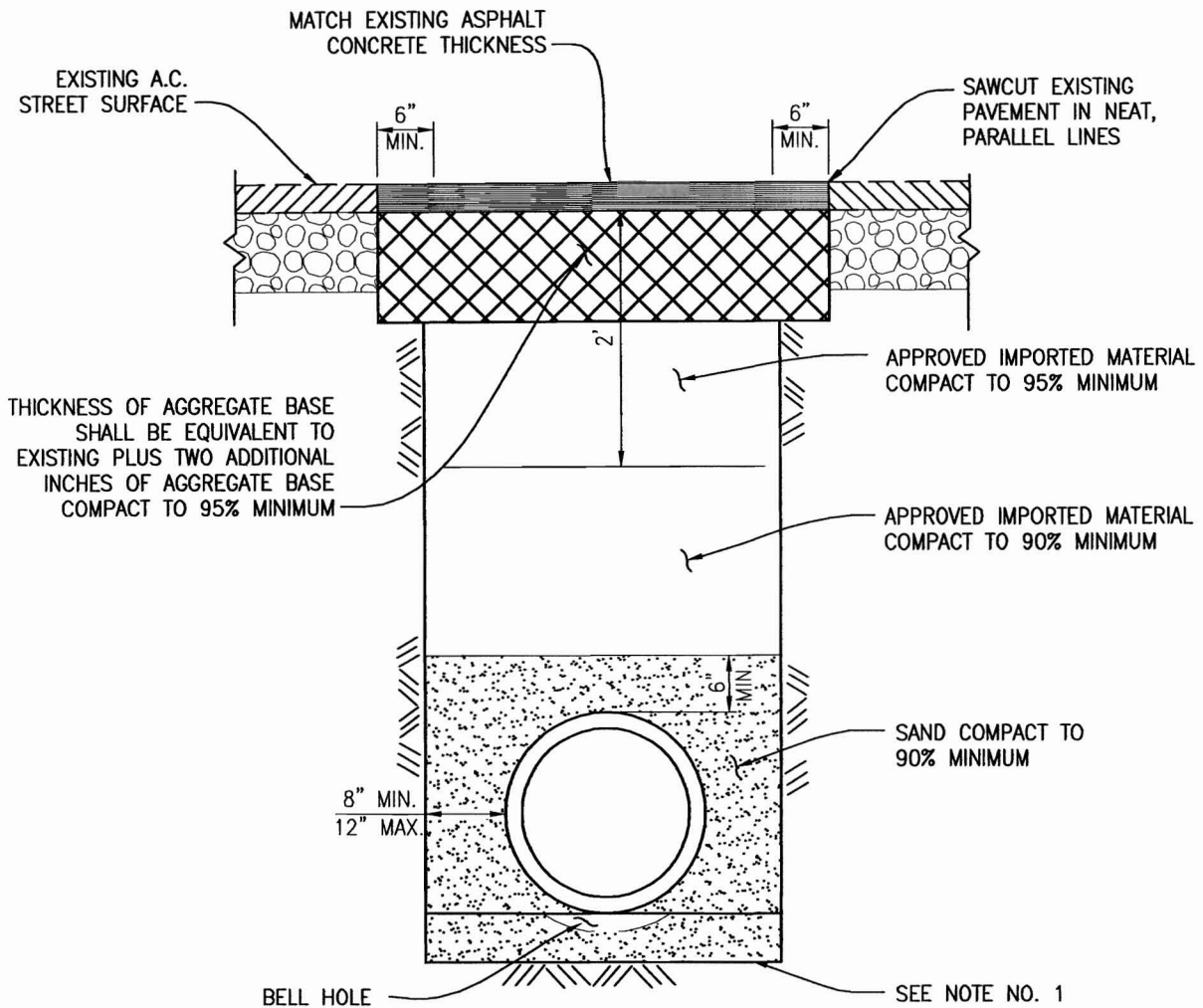
San R. Kelly

DATE:

7/14/09

STD. DETAIL

6 - H



NOTES:

1. IF BOTTOM OF TRENCH IS SOFT OR UNSTABLE, IT SHALL BE OVEREXCAVATED A MINIMUM OF 1 FOOT BELOW GRADE AND BACKFILLED WITH APPROVED IMPORTED MATERIAL.
2. SAND SHALL CONFORM TO THE FOLLOWING GRADING REQUIREMENTS:

SIEVE SIZE	PERCENTAGE PASSING
#4	100
#16	0-70
#30	0-15
#200	0-5

3. TRENCHES NOT IN PAVED AREAS SHALL BE RESTORED TO MATCH EXISTING SURFACE CONDITIONS.
4. SOIL DENSITIES ARE EXPRESSED AS A PERCENTAGE OF MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D 1557.

CITY OF NEWMAN IMPROVEMENT STANDARDS



RIGID WALL PIPE BACKFILL

APPROVED BY:

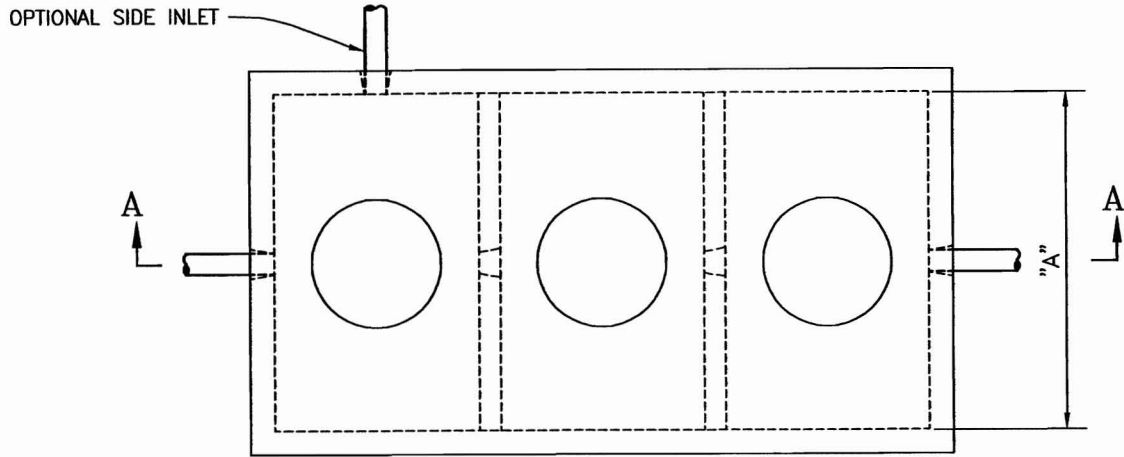
G. L. [Signature]

DATE:

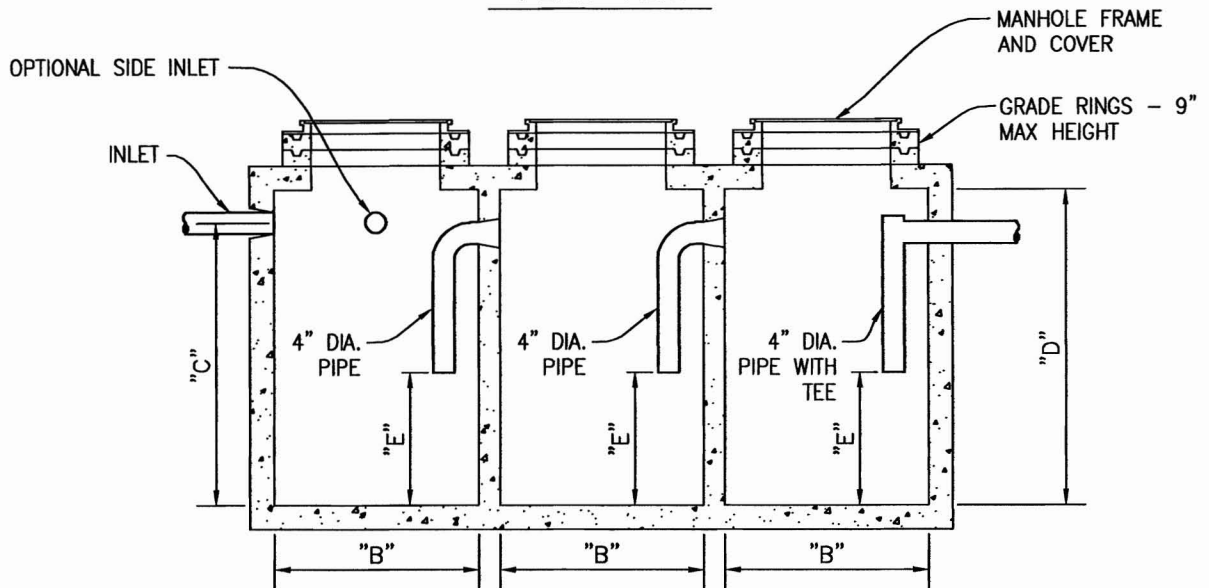
7/14/09

STD. DETAIL

6 - 1



PLAN VIEW



SECTION "A-A"

NOTES:

1. GREASE AND OIL INTERCEPTOR SHOWN IS TO PROTECT PUBLIC IMPROVEMENTS NOT PRIVATE IMPROVEMENTS. SEPARATE KITCHEN GREASE INTERCEPTOR MAY BE REQUIRED.
2. PROVIDE CLEANOUTS AND VENTS AS REQUIRED BY PLUMBING CODE.
3. WALL THICKNESS AND REINFORCING STEEL FOR INTERCEPTORS TO BE DETERMINED BY MANUFACTURER.

CAPACITY GALLONS	DIMENSIONS				
	A	B	C	D	E
1,000	3'-3"	3'-2"	4'-5"	4'-3"	2'-0"
1,500	5'-0"	3'-2"	4'-5"	4'-3"	2'-0"

CITY OF NEWMAN IMPROVEMENT STANDARDS



GREASE & OIL INTERCEPTOR

APPROVED BY:

Garland

DATE:

7/14/09

STD. DETAIL

6 - J

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PAGE _____ OF _____

DESIGNED BY: _____ DATE: _____

CHECKED BY: _____ DATE: _____

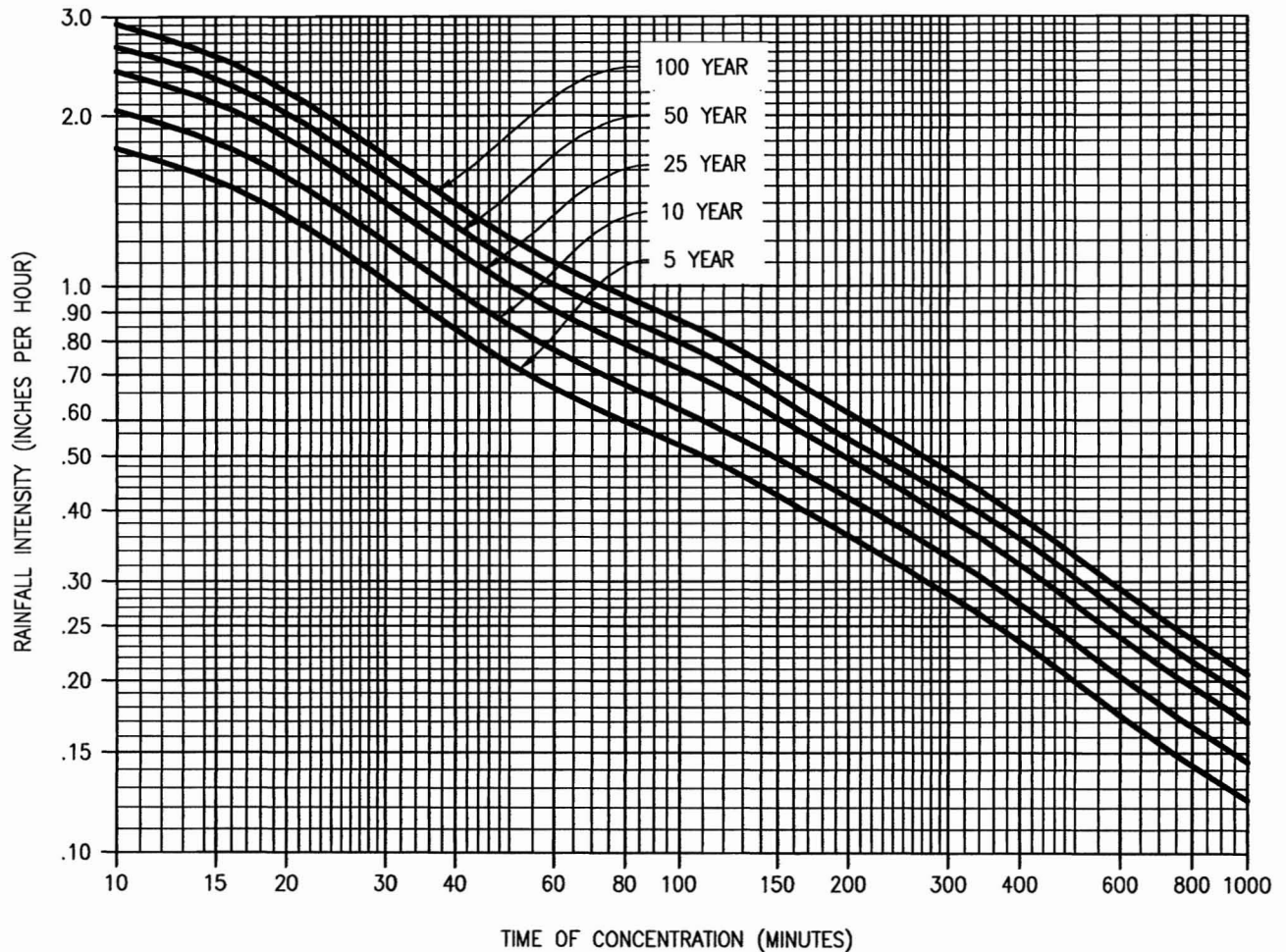
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APPROVED B

APPROVED BY: Ga. D. R. [Signature] DATE: 7/14/09

STD. DETAIL

7 - A



NOTES:

1. ALL STORM DRAINAGE PIPING SHALL BE DESIGNED TO HANDLE A STORM WITH A MINIMUM RETURN INTERVAL OF 10 YEARS.
2. ROOF TO GUTTER TIME SHALL BE ASSUMED TO BE 20 MINUTES.
3. THE 10 YEAR-24 HOUR RAINFALL IS 2.55 INCHES.
4. THE 50 YEAR-24 HOUR RAINFALL IS 3.32 INCHES.
5. THE 100 YEAR-24 HOUR RAINFALL IS 3.63 INCHES

RUNOFF COEFFICIENTS	
ZONING	"C"
R1, R2	0.50
R3	0.60
COMMERCIAL	0.80
INDUSTRIAL	0.70

CITY OF NEWMAN IMPROVEMENT STANDARDS



RAIN INTENSITY FREQUENCY CURVES

APPROVED BY:

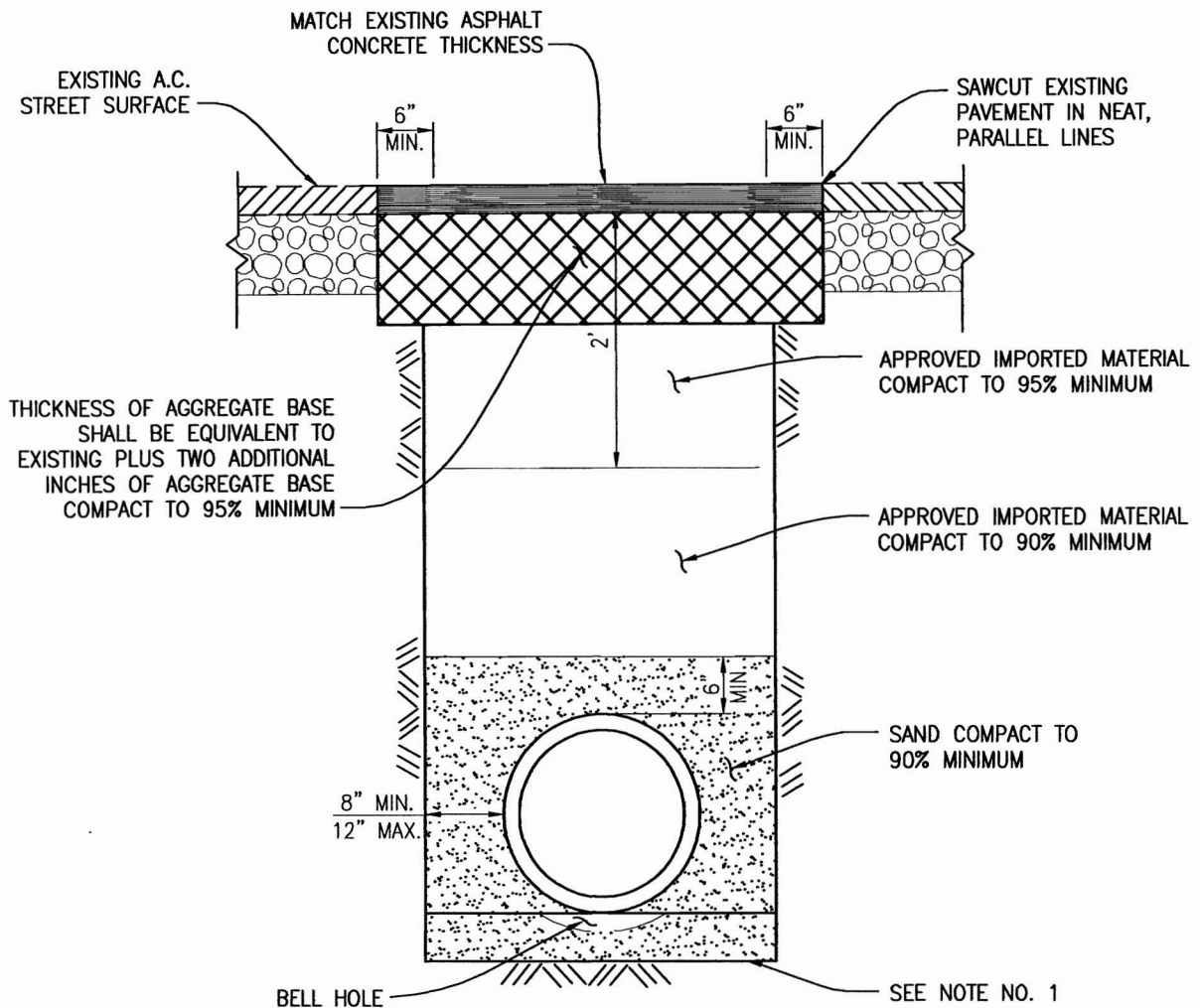
Garrett

DATE:

7/14/09

STD. DETAIL

7-B



NOTES:

1. IF BOTTOM OF TRENCH IS SOFT OR UNSTABLE, IT SHALL BE OVEREXCAVATED A MINIMUM OF 1 FOOT BELOW GRADE AND BACKFILLED WITH APPROVED IMPORTED MATERIAL.
2. SAND SHALL CONFORM TO THE FOLLOWING GRADING REQUIREMENTS:

SIEVE SIZE	PERCENTAGE PASSING
#4	100
#16	0-70
#30	0-15
#200	0-5

3. TRENCHES NOT IN PAVED AREAS SHALL BE RESTORED TO MATCH EXISTING SURFACE CONDITIONS.
4. SOIL DENSITIES ARE EXPRESSED AS A PERCENTAGE OF MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D 1557.

CITY OF NEWMAN IMPROVEMENT STANDARDS



RIGID WALL PIPE BACKFILL

APPROVED BY:

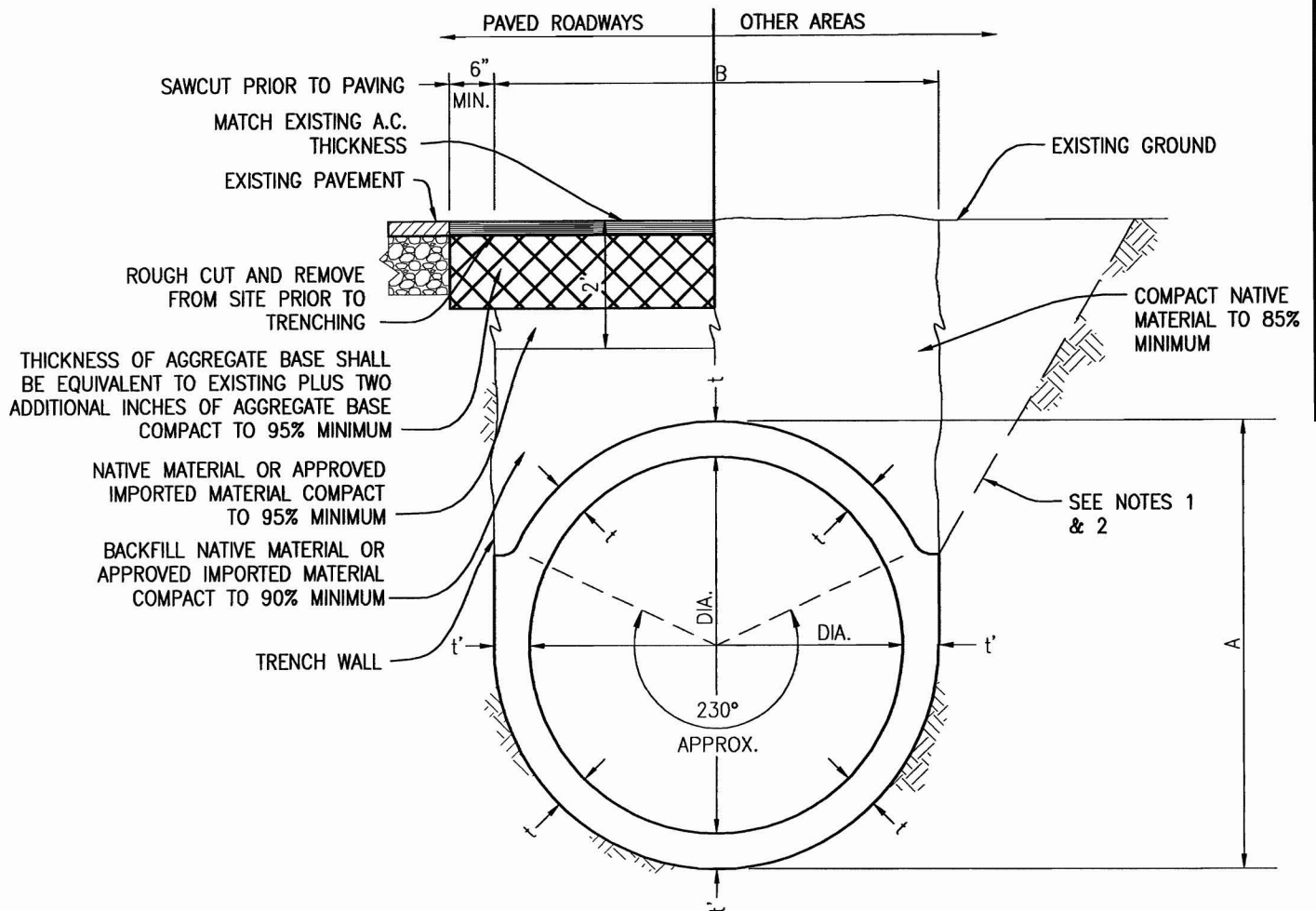
Gar R. Bush

DATE:

7/14/09

STD. DETAIL

7 - C



DIA.	A	MIN. B	t	t'
30"	36"	37"	3"	3"
36"	43"	44"	3 1/2"	3 1/2"
42"	50"	51"	4"	4"
48"	58"	59"	5"	5"
54"	65"	66"	5 1/2"	5 1/2"
60"	72"	73"	6"	6"
66"	79"	80"	6 1/2"	6 1/2"
72"	86"	87"	7"	7"
84"	100"	101"	8"	8"
96"	114"	115"	9"	9"

**TYPICAL PIPE SECTION
30" THRU 96"**

NOTES:

1. THE CONTRACTOR SHALL SUBMIT A DETAILED PLAN TO THE CITY ENGINEER PRIOR TO EXCAVATION, SHOWING DESIGN OF SHORING, BRACING, SLOPING OR OTHER PROVISIONS TO BE MADE FOR WORKER PROTECTION, IN ACCORDANCE WITH SECTION 6422 OF THE LABOR CODE OF CALIFORNIA.
2. THE MINIMUM REQUIRED WORKER PROTECTION SHALL BE AS DESCRIBED IN THE CONSTRUCTION SAFETY ORDERS OF THE DIVISION OF INDUSTRIAL SAFETY. VARIANCES THEREFROM SHALL BE PREPARED AND SIGNED BY A REGISTERED CIVIL ENGINEER OF THE STATE OF CALIFORNIA.
3. TRENCHES IN EXISTING PAVED AREAS SHALL BE EXCAVATED VERTICALLY, OR TO THE MOST NARROW PRACTICAL WIDTH AS SOIL CONDITIONS WILL PERMIT.
4. SOIL DENSITIES ARE EXPRESSED AS A PERCENTAGE OF MAXIMUM DRY DENSITY IN ACCORDANCE WITH ASTM D 1557.

CITY OF NEWMAN IMPROVEMENT STANDARDS



C.I.P.P. STORM DRAIN AND TRENCH

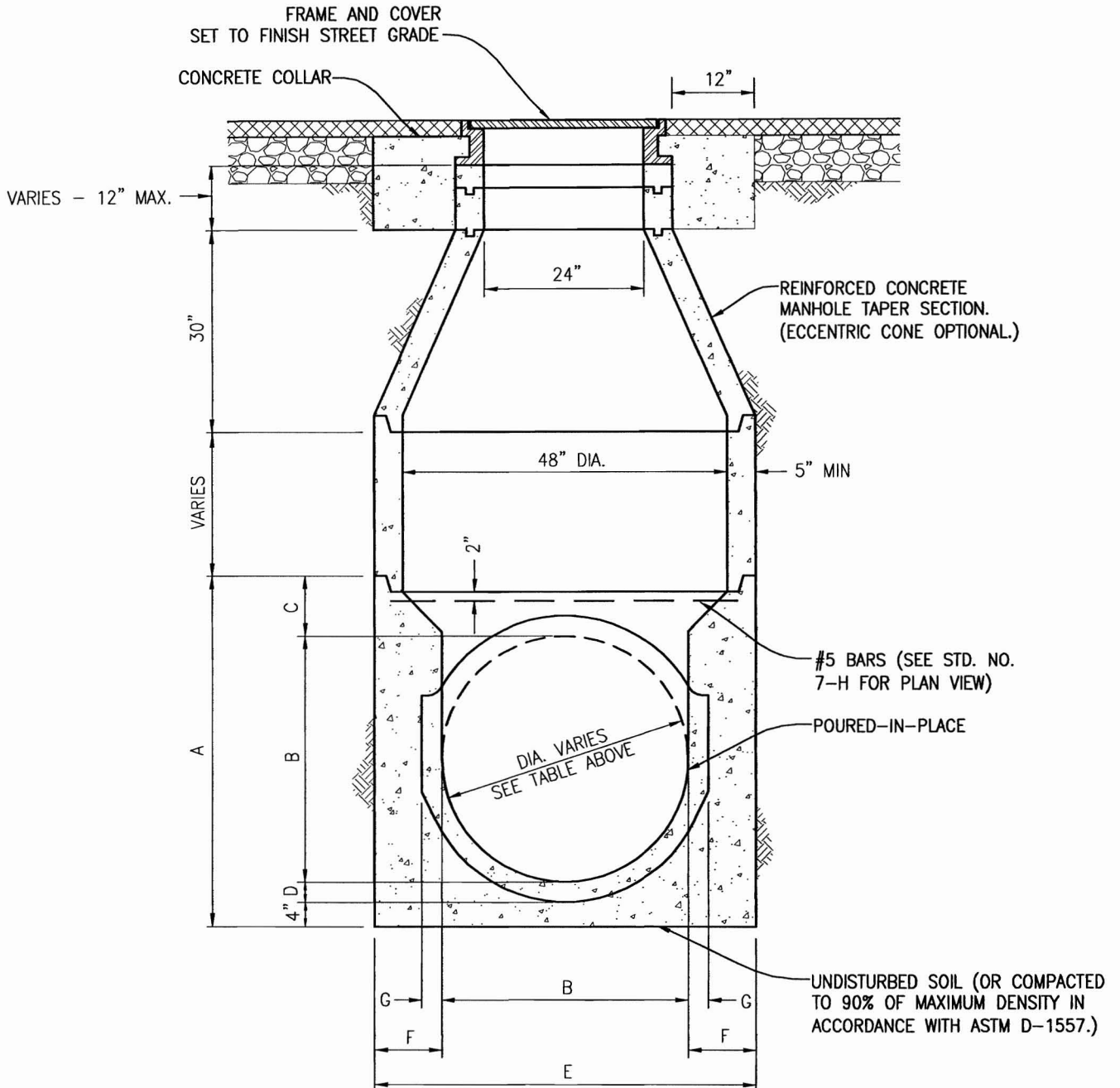
APPROVED BY: *Eric R. B. [Signature]*

DATE: 7/14/09

STD. DETAIL

7 - D

DIA.	A	B	C	D	E	F	MIN. G
30"	46"	30"	9"	3"	56"	13"	3"
36"	53"	36"	9 1/2"	3 1/2"	56"	10"	3 1/2"
42"	60"	42"	10"	4"	60"	9"	4"
48"	68"	48"	11"	5"	66"	9"	5"



CITY OF NEWMAN IMPROVEMENT STANDARDS

STORM DRAIN MANHOLE FOR 30" - 48" C.I.P.P.



APPROVED BY:

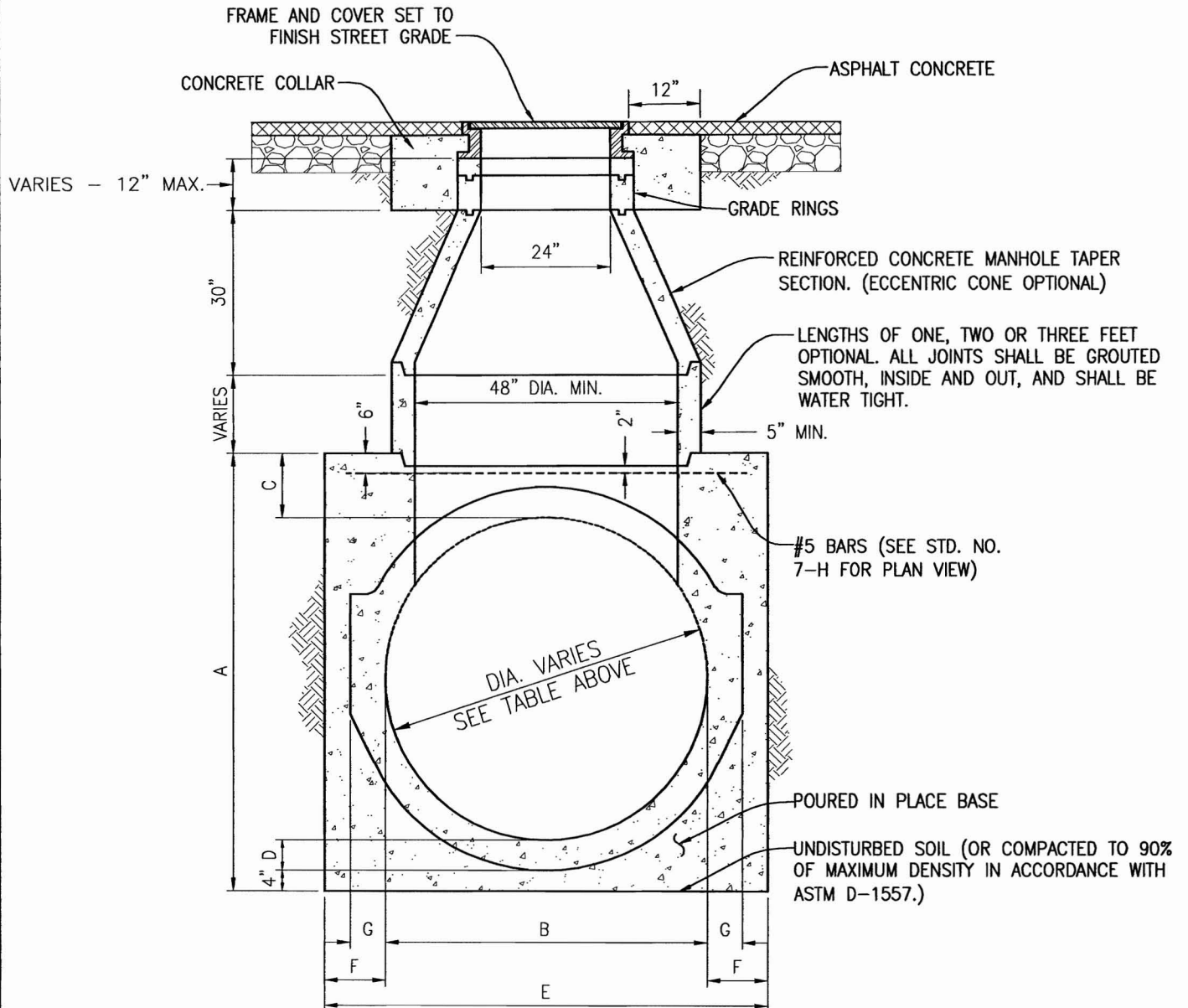
G. R. [Signature]

DATE: 7/14/09

STD. DETAIL

7 - E

DIA.	A	B	C	D	E	F	MIN. G
54"	75"	54"	11 1/2"	5 1/2"	75"	10 1/2"	5 1/2"
60"	82"	60"	12"	6"	82"	11"	6"
66"	89"	66"	12 1/2"	6 1/2"	89"	11 1/2"	6 1/2"
72"	96"	72"	13"	7"	96"	12"	7"
84"	110"	84"	14"	8"	110"	13"	8"
96"	124"	96"	15"	9"	124"	14"	9"



CITY OF NEWMAN IMPROVEMENT STANDARDS



STORM DRAIN MANHOLE FOR 54" - 96" C.I.P.P.

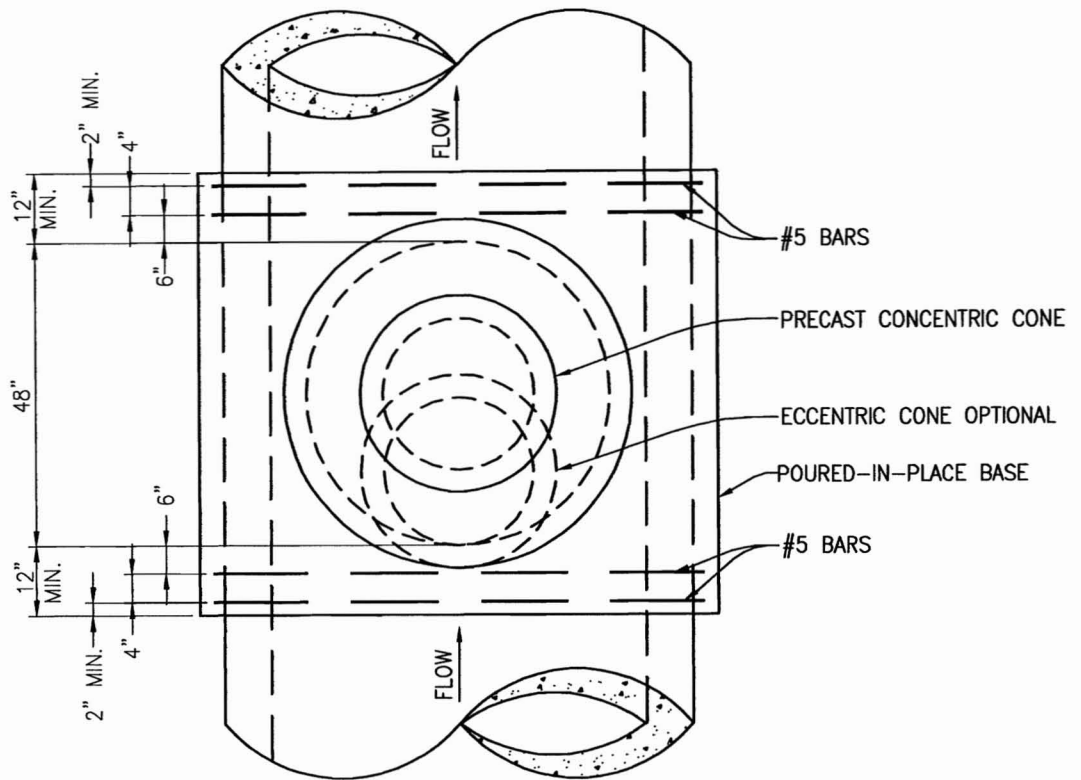
APPROVED BY:

G. L. Bush

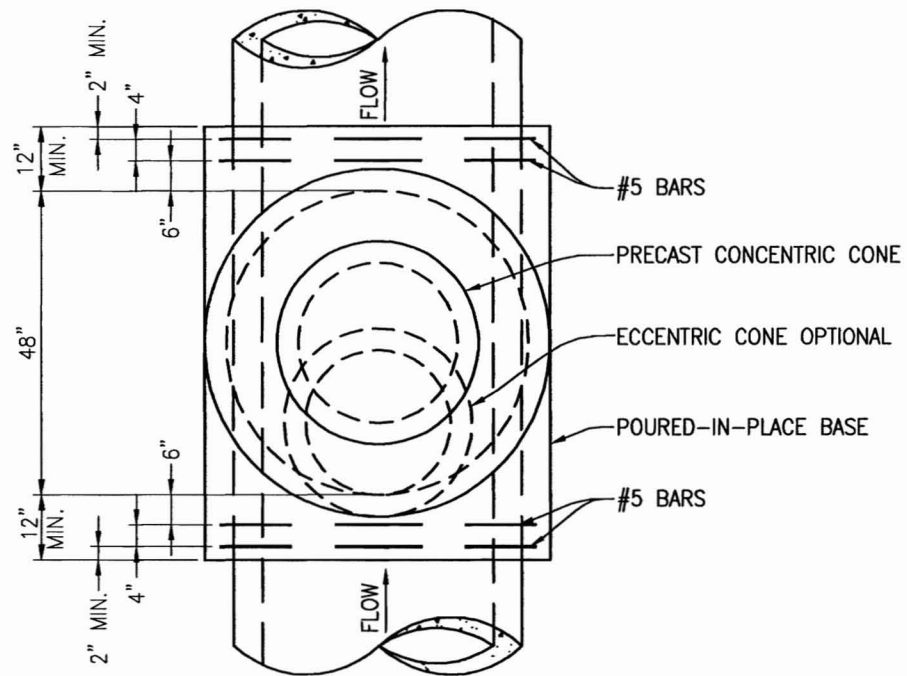
DATE: 7/14/09

STD. DETAIL

7 - F



MANHOLE FOR 54"-96" C.I.P.P.



MANHOLE FOR 30"-48" C.I.P.P.

CITY OF NEWMAN IMPROVEMENT STANDARDS



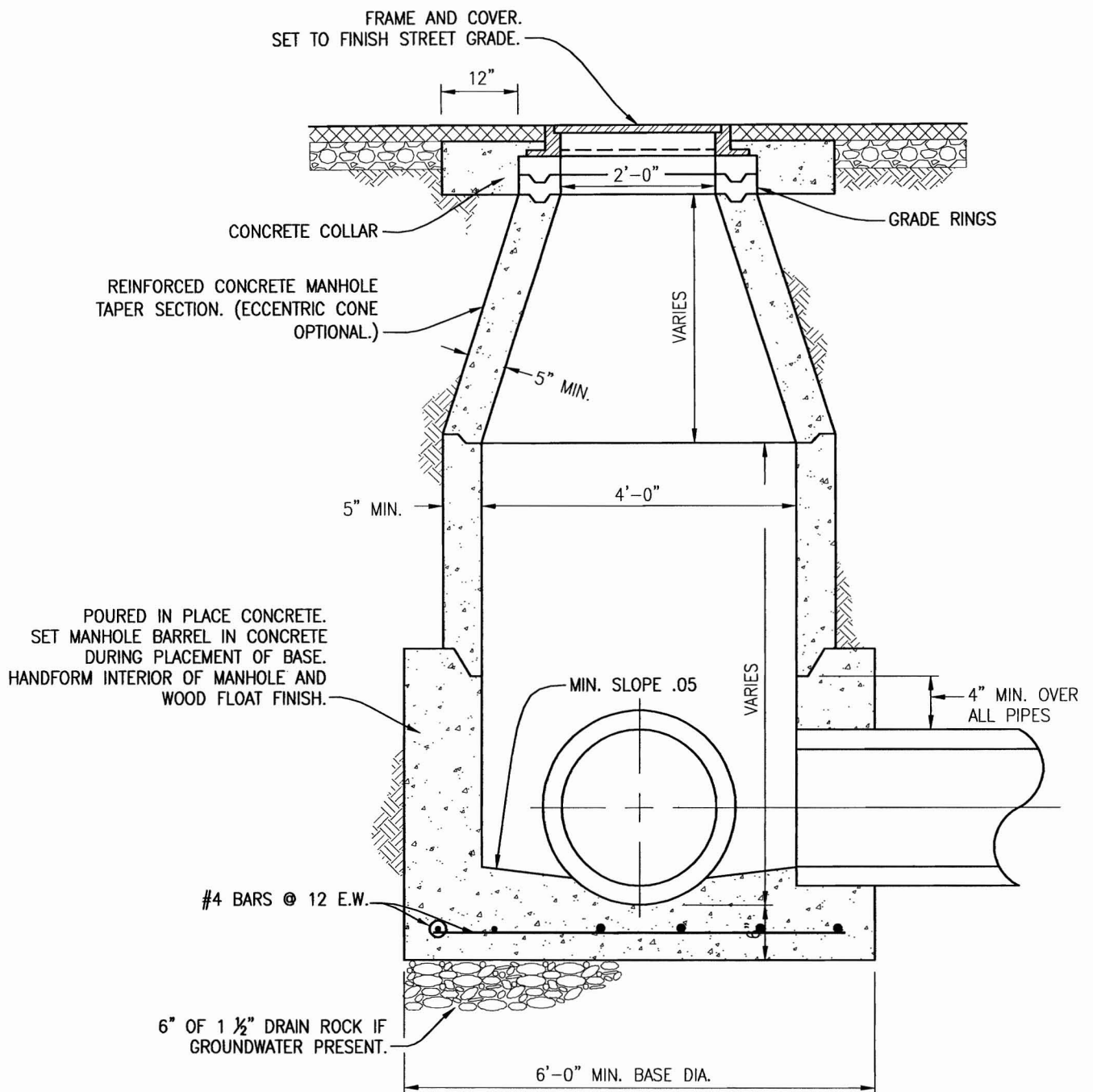
STORM DRAIN MANHOLE PLAN VIEW (C.I.P.P.)

APPROVED BY: *Geo R. Lynch*

DATE: 7/14/09

STD. DETAIL

7 - G



CITY OF NEWMAN IMPROVEMENT STANDARDS



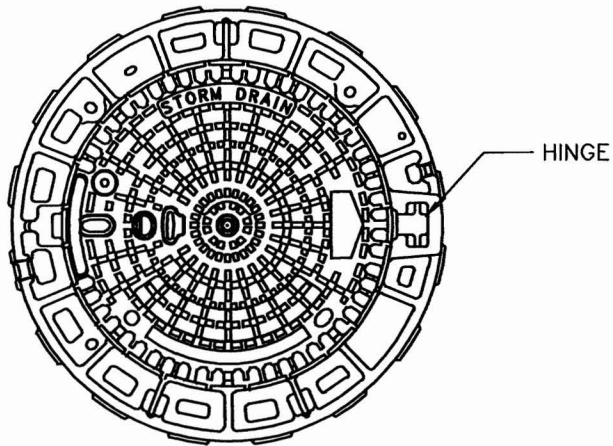
STORM DRAIN MANHOLE

APPROVED BY: *Ga R. Lynch*

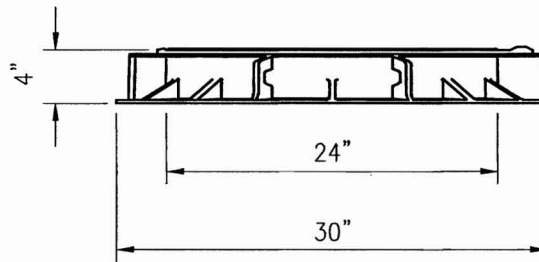
DATE: *7/14/09*

STD. DETAIL

7 - H



PLAN



ELEVATION

NOTES:

1. CERTAINTED PAMREX MANHOLE FRAME AND COVER SHALL BE UTILIZED.
MINIMUM WEIGHT OF DUCTILE IRON FRAME AND COVER 195 LBS.

Jul 08, 2009 - 3:44pm
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CITY OF NEWMAN IMPROVEMENT STANDARDS

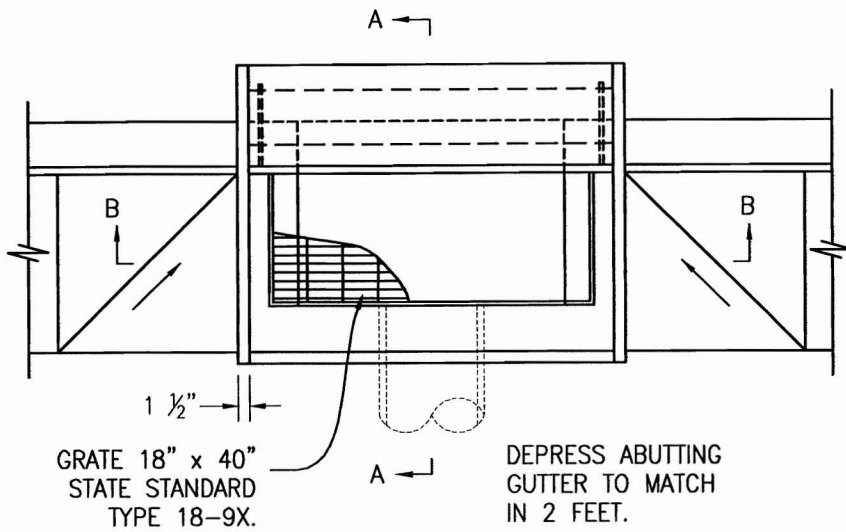
PAMREX MANHOLE FRAME AND COVER

APPROVED BY: *G. R. B.*

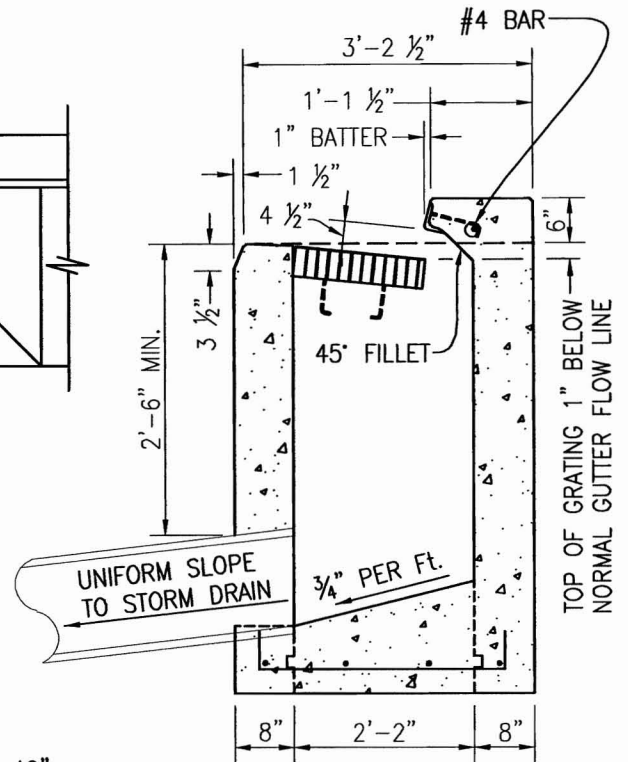
DATE: *7/14/09*

STD. DETAIL

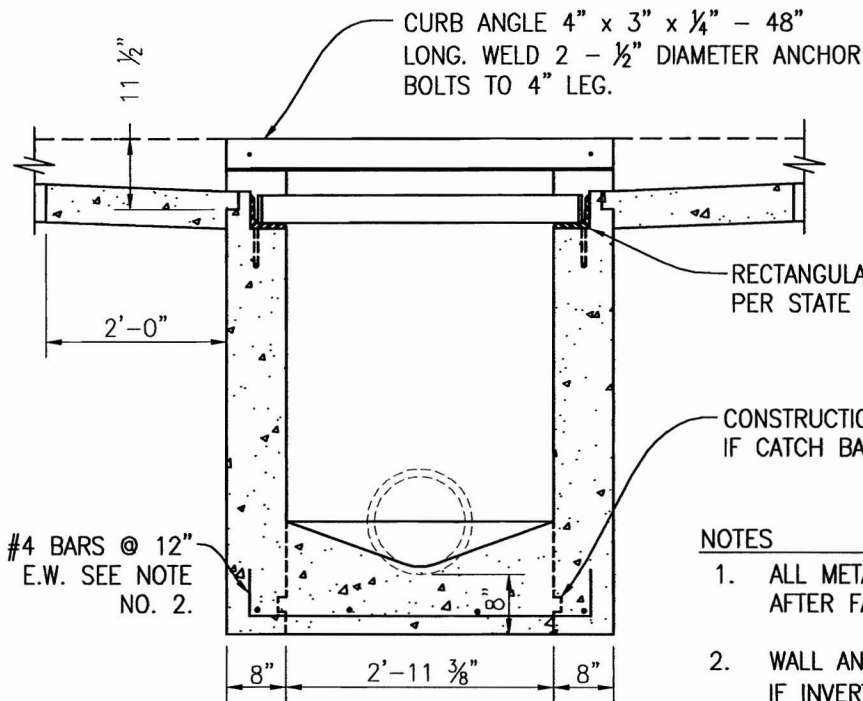
7 - 1



PLAN



SECTION A-A



SECTION B-B

NOTES

1. ALL METAL SHALL BE HOT DIP GALVANIZED AFTER FABRICATION.
2. WALL AND FLOOR REINFORCING NOT REQUIRED IF INVERT LESS THAN 8' DEEP, OTHERWISE USE No. 4 BARS AT 12" EACH WAY, CENTER IN WALLS.

CITY OF NEWMAN IMPROVEMENT STANDARDS

TYPE 'A' CATCH BASIN

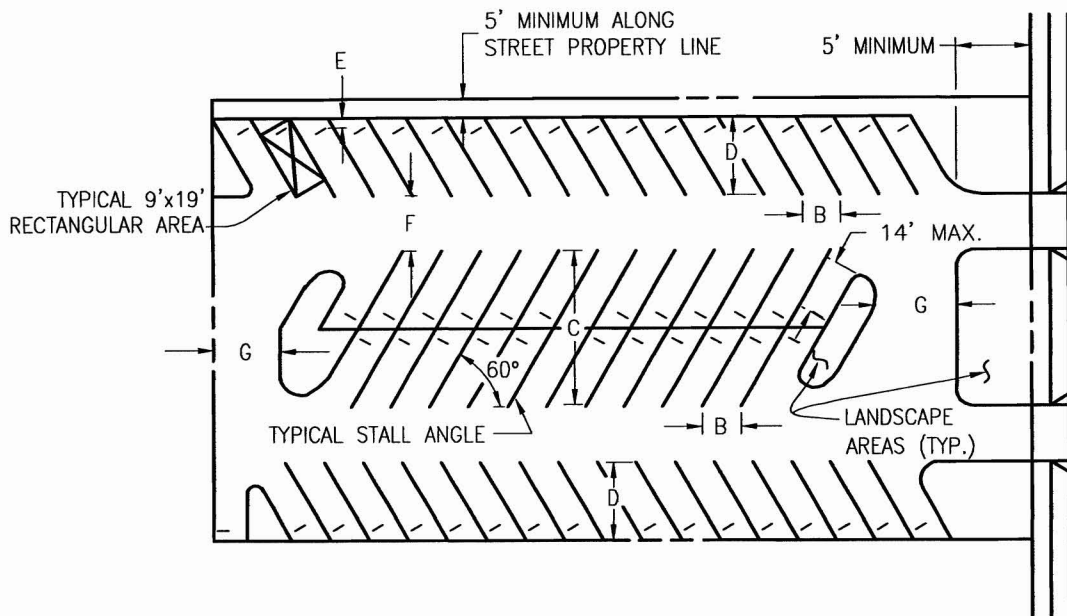


APPROVED BY: *Gab. R.*

DATE: 7/14/09

STD. DETAIL

7 - J



PARKING ANGLE TO DRIVEWAY		45°	60°	70°	80°	90°
STALL WIDTH	B	12.7'	10.4'	9.6'	9.1'	9.0'
STALL DEPTH	C	33.5'	37.5'	38.7'	39.0'	38.0'
STALL DEPTH	D	19.8'	21.0'	20.9'	20.3'	19.0'
DRIVEWAY WIDTH	F	13.0'	18.0'	19.0'	24.0'	25.0'

NOTES:

1. ANY DRIVEWAY USED FOR BOTH INGRESS AND EGRESS TO AND FROM A PARKING LOT AND NOT DIRECTLY SERVING PARKING STALLS SHALL HAVE A MINIMUM WIDTH OF TWENTY FOUR (24) FEET. ANY DRIVEWAY USED ONLY FOR EITHER INGRESS OR EGRESS TO OR FROM A PARKING LOT AND NOT DIRECTLY SERVING PARKING STALLS SHALL BE A MINIMUM OF TWELVE (12) FEET IN WIDTH.
2. THE ABOVE TYPICAL PARKING LOT DIAGRAM, PARKING STALL AND DRIVEWAY DIMENSION TABLE SHALL DETERMINE THE MINIMUM REQUIREMENTS FOR A PARKING LOT PLAN.
3. THE MINIMUM DRIVEWAY WIDTH (F) AT ANY PARKING STALL ANGLE LESS THAN 45°, INCLUDING PARALLEL STALLS, IS 8.8 FEET.
4. THE TURNAROUND OR END DRIVEWAY WIDTH (G) SHALL BE A MINIMUM OF 18 FEET.
5. THE WHEELSTOP SET BACK DIMENSION (E) SHALL BE A MINIMUM OF 2 FEET FOR ANY PARKING PLAN (3 FEET MINIMUM REQUIRED FROM PROPERTY LINE).

CITY OF NEWMAN IMPROVEMENT STANDARDS



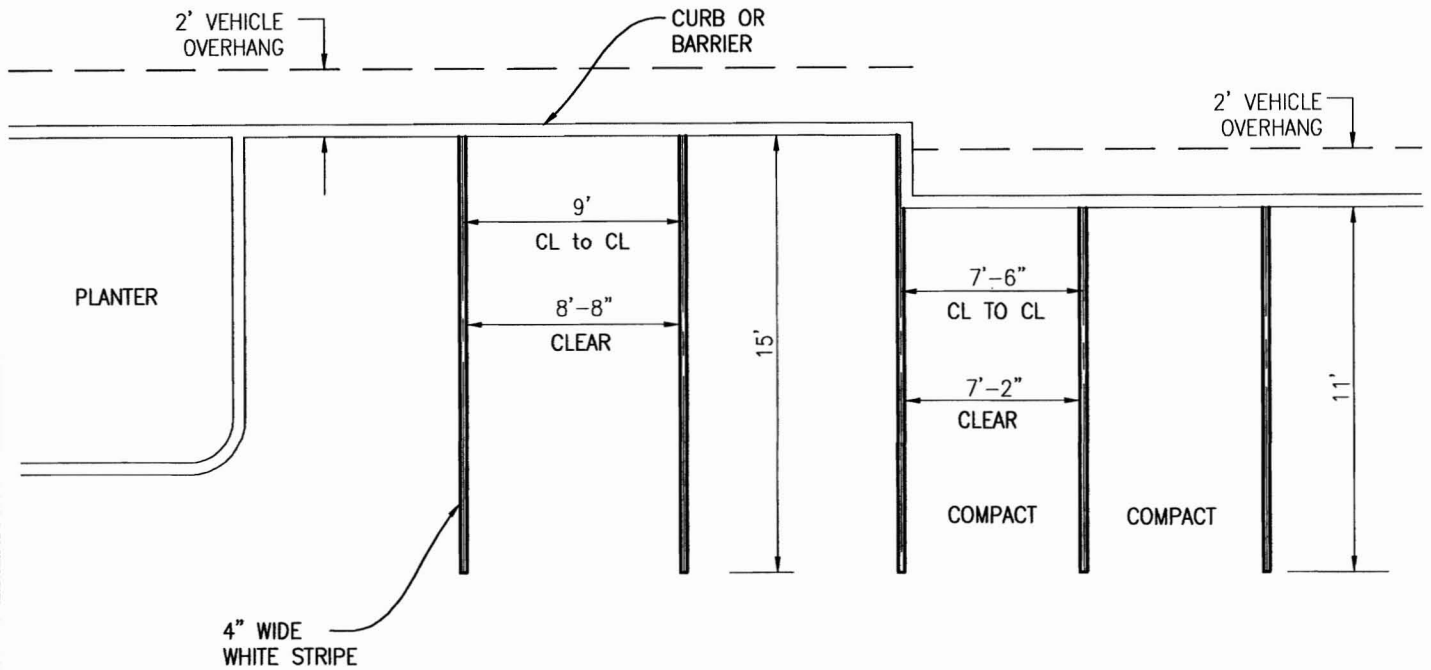
OFF-STREET PARKING STANDARDS

APPROVED BY: *[Signature]*

DATE: 7/14/09

STD. DETAIL

8 - A



NOTE:

STALL STRIPE IS SHORTER THAN LENGTH OF STALL.

CITY OF NEWMAN IMPROVEMENT STANDARDS

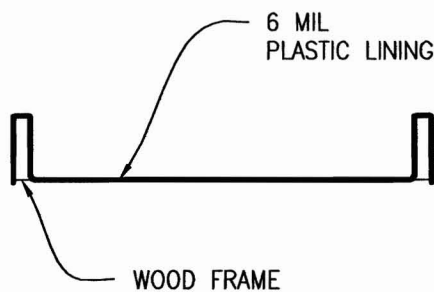
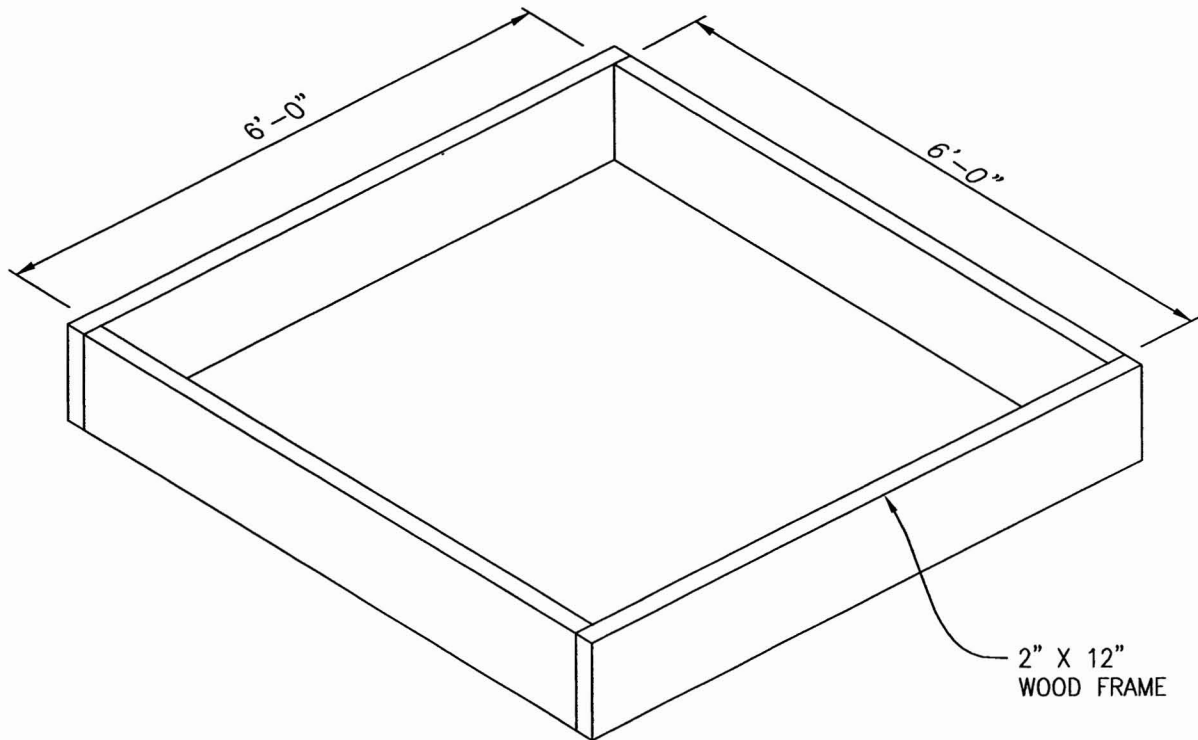


TYPICAL PARKING STALL STRIPING

APPROVED BY: *Grant R. Byler* DATE: *7/14/09*

STD. DETAIL

8 - B



NOTES:

1. INSTALL 6 MIL PLASTIC LINER IN WOOD FRAME.
2. FOR USE IN WASHING OUT CONCRETE TRAILERS. FOLLOWING EVAPORATION OF WATER, BREAK-UP CONCRETE AND FRAME FOR DISPOSAL.
3. SIZE OF CONCRETE WASHOUT SHALL BE ADJUSTED ACCORDING TO PROJECT NEEDS.

CITY OF NEWMAN IMPROVEMENT STANDARDS



CONCRETE WASHOUT - SMALL OPERATIONS

APPROVED BY: *Geo R. Byrbe* DATE: 7/14/09

STD. DETAIL

8 - C